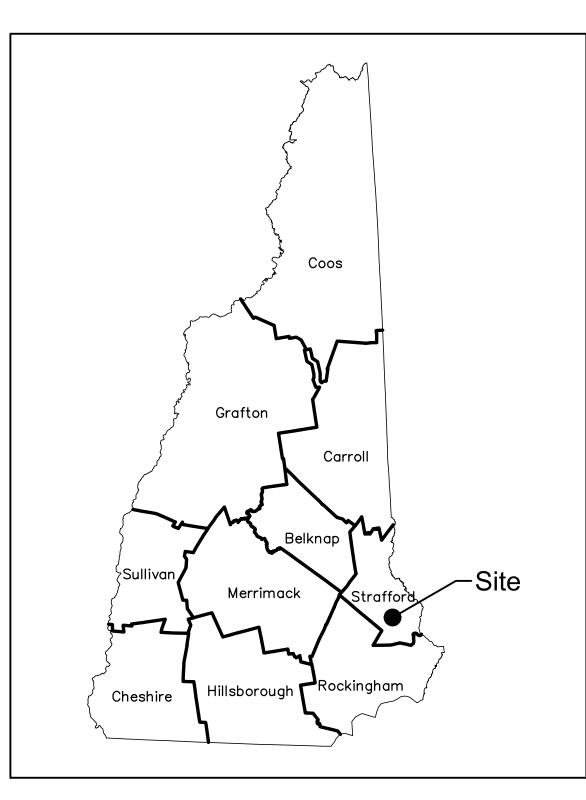
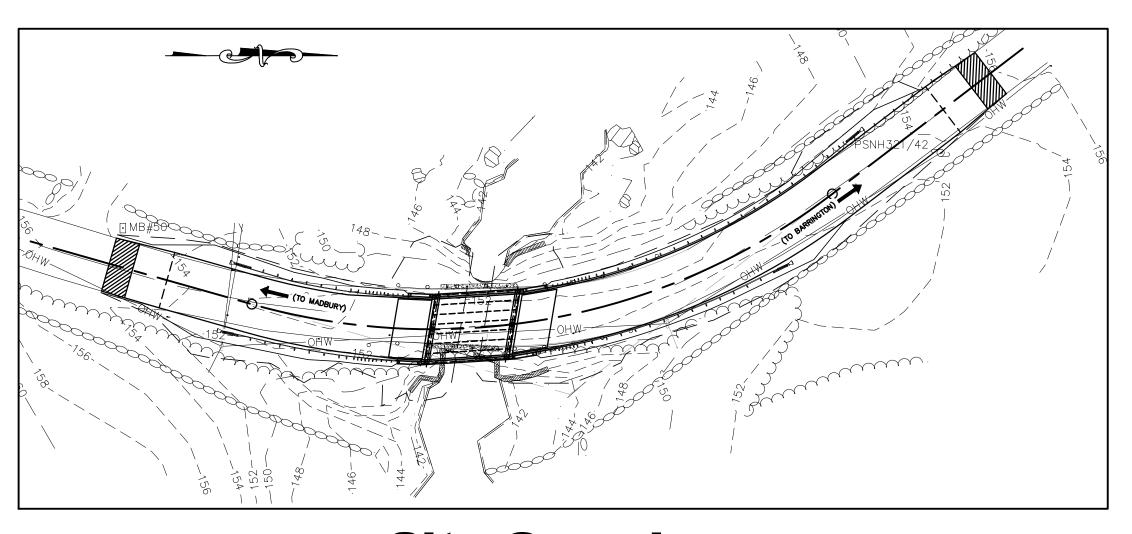
Town Of Madbury, New Hampshire Nute Road Bellamy River Crossing Replacement Bridge No. 056/072

Final Design Issued for Review July 2020

NHDOT Project No. 24226



Locus Plan



Site Overview

Scale: 1" = 40'

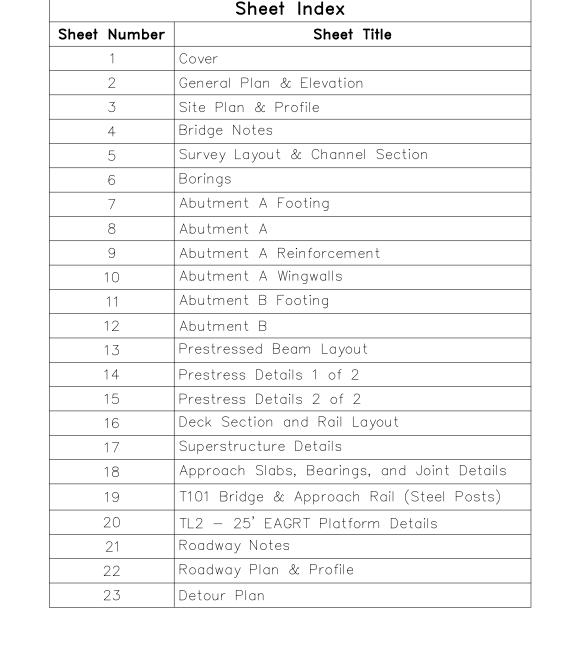
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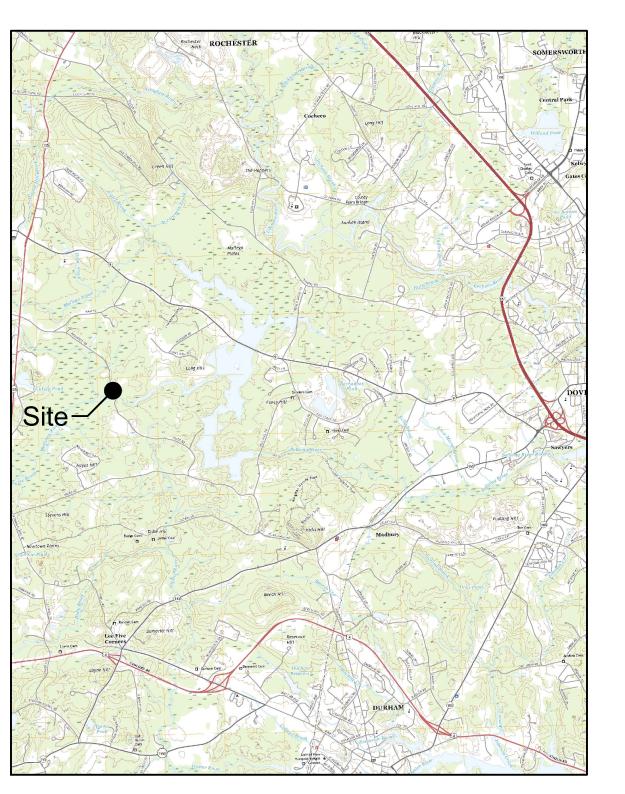
Town of Madbury
13 Town Hall Road
Madbury, New Hampshire 03823

Prepared By:



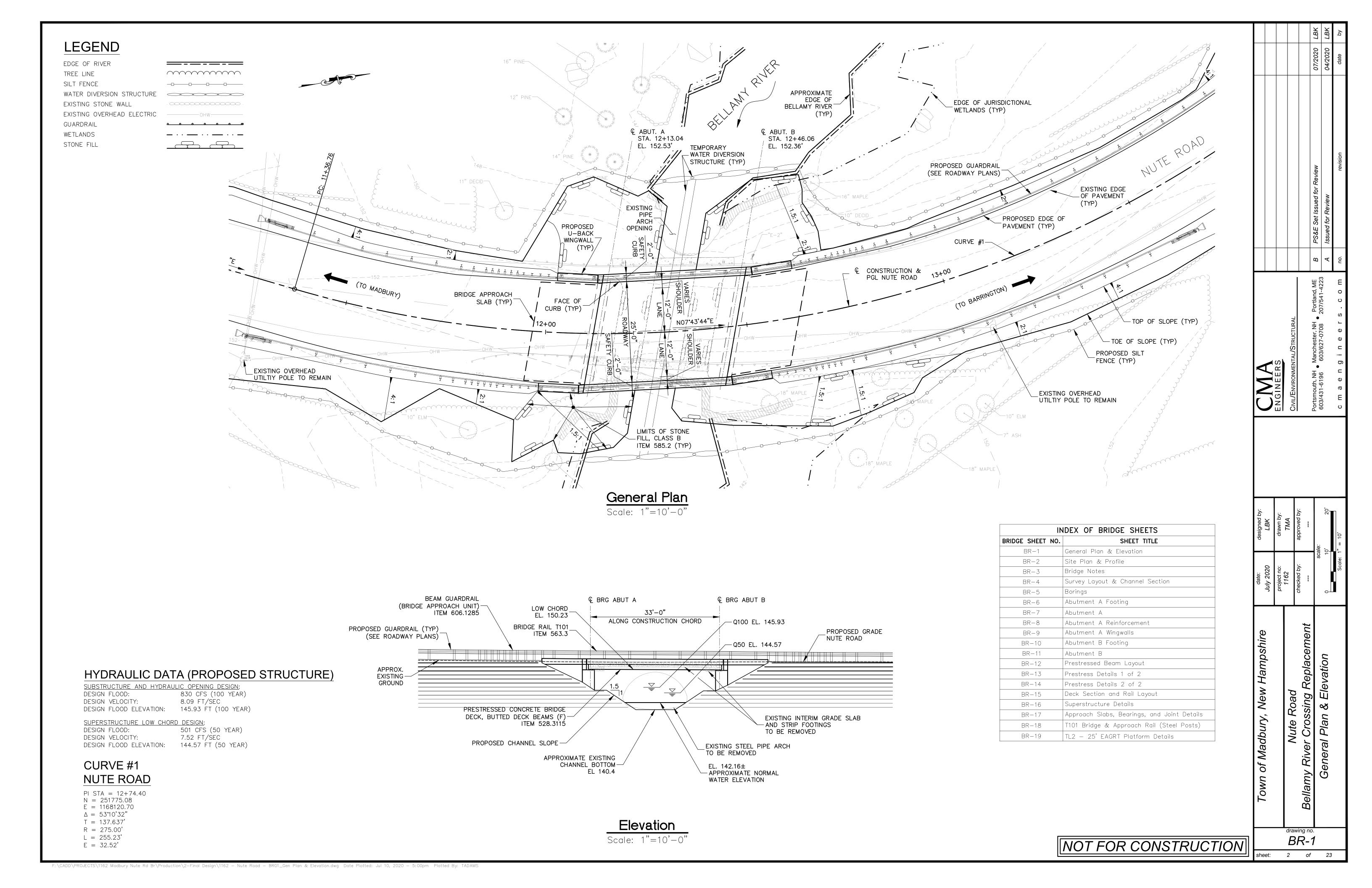
Portsmouth, NH 603/431-6196 Manchester, NH 603/627-0708 Portland, ME 207/541-4223

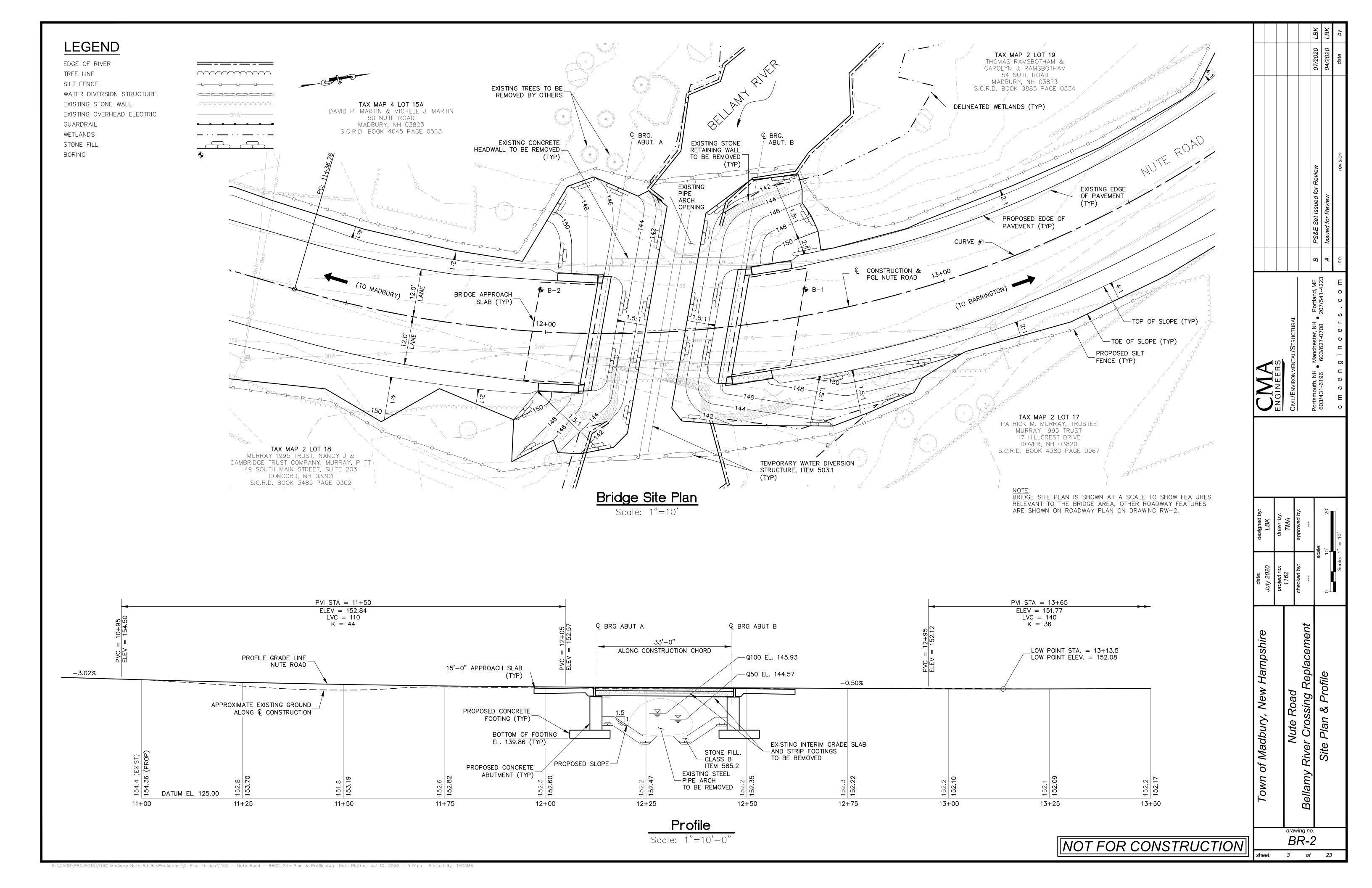




Project Location

Scale: 1" = 1 mile





Structure Notes:

1) DESIGN LOADING: HL-93

2) DESIGN METHOD: LOAD AND RESISTANCE FACTOR DESIGN METHOD (LRFD).

3) SPECIFICATIONS: AASHTO LRFD 2017. NHDOT 2016 STANDARD SPECIFICATIONS AS AMENDED.

4) FOUNDATION DATA: CONCRETE ABUTMENTS SUPPORTED ON SPREAD FOOTINGS, ALLOWABLE BEARING PRESSURE 2 KSF, MINIMUM FROST DEPTH 6.0 FT PERPENDICULAR BETWEEN NEAREST GRADE

AND BOTTOM OF FOOTING.

5) REINFORCING STEEL: AASHTO M31 (ASTM A615) GRADE 60. REINFORCING STEEL IN THE DECK OVERLAY, DECK BEAMS, APPROACH SLABS, AND BRUSH CURBS SHALL BE EPOXY COATED.

6) CONCRETE: BRIDGE DECK OVERLAY, CURBS, WINGWALL COPINGS AND APPROACH SLABS = 4 KSI

ABUTMENTS AND WINGWALLS = 3 KSI FOOTINGS = 3 KSI PRESTRESSED DECK BEAMS = 6 KSI, RELEASE = 4.8 KSI

7) POST-TENSIONING STEEL: 0.6" Ø SEVEN-WIRE STRAND CONFORMING TO AASHTO M203 (ASTM A416) GRADE 270, LOW RELAXATION.

8) PRESTRESSING STEEL: 0.6" Ø SEVEN-WIRE STRAND CONFORMING TO AASHTO M203 (ASTM A416) GRADE 270, LOW RELAXATION.

9) SEISMIC PERFORMANCE ZONE 1: A = 0.1

10) ALL EXISTING BRONZE DISCS REPRESENTING STATE BENCHMARKS OR SURVEY TRIANGULATION POINTS MUST NOT BE DISTURBED. WHEN THE WORK CALLED FOR INVOLVES DISTURBING A BRONZE DISK, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SUFFICIENTLY IN ADVANCE OF THE WORK TO PERMIT THE TOWN, STATE OR AGENCY HAVING JURISDICTION TO TEMPORARILY RELOCATE THE AFFECTED MARKER.

11) MAINTENANCE OF TRAFFIC: ROAD CLOSURE AND DETOUR; SEE ROADWAY SHEET RW-3.

12) FOR STRUCTURE LAYOUT SEE BRIDGE SHEET BR-4.

Bridge Removal Notes:

1) PLANS OF THE EXISTING STEEL PIPE ARCH AND THE INTERIM REPAIR INSTALLED IN 2012 ARE INCLUDED IN THE CONTRACT DOCUMENTS. THIS REPAIR INCLUDED THE INSTALLATION OF TWO GRADE BEAM FOOTINGS PERPENDICULAR TO THE ROAD WHICH SUPPORT A SLAB ON GRADE SPANNING OVER THE CULVERT.

2) ITEM 502, REMOVAL OF EXISTING BRIDGE STRUCTURE, SHALL INCLUDE REMOVAL OF THE ENTIRE CORRUGATED METAL PIPE ARCH, CONCRETE HEADWALLS, STONE MASONRY RETAINING WALLS AND CUTOFF WALLS, AND 2012 INTERIM REPAIR STRUCTURES (GRADE BEAMS AND SLAB) AS DESCRIBED WITHIN THESE PLANS. SEE SPECIAL CONDITIONS FOR MATERIALS TO BE SALVAGED TO THE TOWN. ALL OTHER EXISTING BRIDGE MATERIALS SHALL BECOME PROPERTY OF THE CONTRACTOR.

Water Diversion Structure Notes:

1) ITEM 503.1, WATER DIVERSION STRUCTURES, MAY BE REQUIRED FOR REMOVAL OF THE EXISTING STRUCTURE AND THE CONSTRUCTION OF THE ABUTMENTS DURING HIGH FLOW CONDITIONS. THE CONTRACTOR SHALL SUBMIT THE DIVERSION STRUCTURE TYPE, DESIGN, AND PROPOSED METHOD OF CONSTRUCTION TO THE ENGINEER IN ACCORDANCE WITH SECTION 105.02 OF THE NHDOT STANDARD SPECIFICATIONS. WATER DIVERSION STRUCTURE SUBMITTALS SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NEW HAMPSHIRE.

2) WATER DIVERSION STRUCTURE LIMITS SHOWN ON THE PLANS ARE APPROXIMATE. THE CONTRACTOR SHALL DETERMINE THE REQUIRED LIMITS, IN ACCORDANCE WITH THE ENVIRONMENTAL PERMIT, TO MAINTAIN A DEWATERED AND ADEQUATELY SUPPORTED EXCAVATION DURING THE CONSTRUCTION.

3) ALL COSTS FOR DESIGN, INSTALLATION AND REMOVAL OF WATER DIVERSION STRUCTURES SHALL BE INCLUDED IN ITEM 503.1.

4) CONTROL OF WATER WITHIN THE COFFERDAMS SHALL BE CONDUCTED IN SUCH A MANNER AS TO PREVENT DISTURBANCE OF THE BEARING SOIL. PUMPING AREAS SHALL BE LOCATED OUTSIDE THE FOOTING SUPPORT LIMITS AND PROPERLY FILTERED TO PREVENT THE PUMPING OF FINES.

5) ANY FOUNDATION SOIL WEAKENED AS A RESULT OF INSUFFICIENT CARE TAKEN IN MAINTAINING A DEWATERED CONDITION SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL AT THE EXPENSE OF THE CONTRACTOR.

6) DEWATERING SHALL BE CONTINUOUS UNTIL SUBSTRUCTURES ARE BACKFILLED TO THE ELEVATIONS OF THE SURROUNDING WATER TABLE, UNLESS NOTED OTHERWISE.

Natural River Bed Material Notes:

1) THE INTENT OF THE NATURAL RIVER MATERIAL IS TO SIMULATE THE SURFACE CONDITION OF THE EXISTING RIVER PRIOR TO CONSTRUCTION. SURFACE ROUGHNESS, PERMEABILITY, AND PARTICLE SIZE AND DISTRIBUTION SHOULD ALL BE ADDRESSED AS PART OF THIS WORK.

2) ANY EXCAVATED NATURAL RIVER MATERIAL SHALL BE SEPARATED FROM OTHER EXCAVATED MATERIAL, AND STOCKPILED FOR USE LATER IN THE PROJECT.

3) IF IMPORTING MATERIAL IS REQUIRED, CONTRACTOR SHALL FIND A WELL-GRADED MATERIAL TO ROUGHLY MATCH THE MATERIAL FOUND UP AND DOWNSTREAM OF THE PROPOSED CONSTRUCTION. ALL STONES SHALL BE ROUND, WITHOUT FRACTURES, AND WASHED IN WITH SILTY FINE MATERIAL. THE RIVER MATERIAL SHALL NOT ALLOW FOR SUBSURFACE FLOW. ALL COSTS FOR DELIVERY, STOCKPILING, AND PLACEMENT OF THIS MATERIAL SHALL BE SUBSIDIARY TO NHDOT ITEM 585.2.

Boring Notes:

1) BORINGS INDICATED THUS WERE MADE BY SOIL EXPLORATION CORP. IN AUGUST 2018. BLOW COUNTS SHOWN ARE THE NUMBER OF BLOWS REQUIRED TO DRIVE A 2"O.D. STANDARD SPLIT SPOON SAMPLER 6", USING A 140LB WEIGHT FALLING 30".

2) BORINGS ARE FOR DESIGN PURPOSES ONLY, SHOWING CONDITIONS AT THE BORING POINTS ONLY, AND DO NOT NECESSARILY INDICATE MATERIAL TO BE ENCOUNTERED DURING CONSTRUCTION.

3) GROUNDWATER LEVELS NOTED, IF ANY, WERE MEASURED AT THE TIME OF EXPLORATION. THE WATER LEVELS ENCOUNTERED DURING CONSTRUCTION MAY VARY CONSIDERABLY DUE TO PREVAILING CLIMATE, RAINFALL, OR OTHER FACTORS.

Precast Deck Beam Notes:

 THE CONCRETE COMPRESSIVE STRENGTH OF THE PRECAST DECK BEAM UNITS SHALL BE 4800 PSI AT RELEASE AND 6000 PSI AT 28 DAYS.

2) PRESTRESSING STEEL SHALL BE 0.6" Ø UNCOATED SEVEN-WIRE STRAND CONFORMING TO AASHTO M203 (ASTM A416) GRADE 270 LOW RELAXATION. ALL STRANDS SHALL BE PRE-TENSIONED TO 43.9 KIPS PER STRAND (75% INITIAL PULL).

POST-TENSIONING STRANDS SHALL BE COMPLETELY COATED WITH A CORROSION PREVENTATIVE COATING SUCH AS FLO-GUARD, AS MANUFACTURED BY INSTEEL INDUSTRIES, INC., SANDERSON, FL., OR POLYSTRAND, AS MANUFACTURED BY LANG TENDONS, INC., TOUGHKENAMON, PA., OR AN APPROVED EQUAL. IF THE FLO-GUARD COATING SYSTEM IS SUPPLIED, GROUT SHALL BE EXCLUDED FROM THE LATERAL POST-TENSIONING DUCTS DURING GROUTING OF THE SHEAR KEYS BETWEEN THE BEAMS. THE CONTRACTOR'S PROPOSED METHOD FOR EXCLUDING THE GROUT FROM THE POST-TENSIONING DUCTS SHALL BE SUBMITTED WITH THE SHOP DRAWINGS. POST-TENSIONING ANCHORAGE SYSTEM SHALL BE MONO-STRAND CORROSION PROTECTION SYSTEM AS MANUFACTURED BY HAYES INDUSTRIES, INC., HOUSTON TEXAS OR APPROVED EQUAL.

4) TRANSVERSE POST—TENSIONING OF THE PRECAST DECK BEAMS SHALL BE PERFORMED IN ACCORDANCE WITH SECTION 528 OF THE STANDARD SPECIFICATIONS.

5) ALL REINFORCING STEEL FOR THE SUPERSTRUCTURE SHALL CONFORM TO AASHTO M31 (ASTM A615) GRADE 60 AND SHALL BE EPOXY COATED.

6) THE PRECAST DECK BEAM REINFORCING STEEL SHALL HAVE A MINIMUM CLEAR COVER OF 1 1/2" UNLESS OTHERWISE NOTED.

7) THE COST OF PRESTRESSING STRANDS, POST—TENSIONING STRANDS AND ANCHORAGES, AND REINFORCING STEEL CAST INTO THE PRECAST DECK BEAM UNITS SHALL BE SUBSIDIARY TO ITEM 528.3115. ALL OTHER STEEL IN THE SUPERSTRUCTURE SHALL BE PAID UNDER ITEMS 544.3 AND 544.31.

8) LIFTING DEVICES SHALL BE WITHIN 24" OF EACH END OF THE PRECAST DECK BEAM UNITS. COST SHALL BE SUBSIDIARY TO ITEM 528.3115.

9) 1" Ø DRAINS SHALL BE PROVIDED AT THE THE LOW END OF ALL DECK BEAM VOIDS.

10) THE DECK BEAM SHEAR KEYS SHALL BE BLAST CLEANED PRIOR TO SHIPPING.

11) THE TOP SURFACE OF THE DECK BEAMS SHALL BE RAKED TRANSVERSELY TO A $\frac{1}{4}$ " AMPLITUDE.

12) DRILLING INTO THE DECK BEAMS SHALL NOT BE ALLOWED.

13) DIFFERENTIAL CAMBER (AT ERECTION) BETWEEN ADJACENT MEMBERS SHALL BE LIMITED TO 1". VALUES FOR MIDSPAN CAMBER AT TRANSFER SHALL BE DETAILED ON THE SHOP DRAWINGS.

14) PROVIDE INSERTS TO SUPPORT THE CONTRACTOR'S OVERHANG BRACKETS IN THE TOP SLAB ON EXTERIOR UNITS AT THE SPACING REQUESTED BY THE GENERAL CONTRACTOR. INSERTS SHALL BE SHOWN ON THE SHOP DRAWINGS. ALL COSTS SHALL BE SUBSIDIARY TO ITEM 528.3115.

Bridge Deck Notes:

1) CONCRETE MIXES SHALL BE AS FOLLOWS:

CAST-IN-PLACE OVERLAY: ITEM 520.7002, CONCRETE BRIDGE DECK (QC/QA) (F)

BRUSH CURBS: ITEM 520.02,

CONCRETE CLASS AA, ABOVE FOOTINGS (F)

 CONCRETE OVERLAY FINISHING SHALL MEET THE REQUIREMENTS OF 520.3.9.2 UNLESS OTHERWISE APPROVED IN WRITING BY THE ENGINEER.

3) THE CONTRACTOR SHALL NOT DRILL INTO THE PRECAST DECK BEAMS UNLESS APPROVED IN WRITING BY THE ENGINEER.

4) ALL REINFORCEMENT IN THE BRIDGE DECK OVERLAY AND BRUSH CURBS SHALL BE EPOXY COATED AND SHALL BE PAID AS ITEM 544.31, REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED).

5) ALL REINFORCING SHALL BE 2 ½" FROM CONCRETE SURFACES, UNLESS OTHERWISE NOTED.

Approach Slab Notes:

1) ALL CONCRETE FOR THE APPROACH SLABS SHALL BE ITEM 520.03, CONCRETE CLASS AA, APPROACH SLABS (F).

2) ALL REINFORCEMENT IN THE APPROACH SLABS SHALL BE EPOXY COATED AND SHALL BE PAID AS ITEM 544.31, REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED).

3) ALL REINFORCING SHALL BE 2 $\frac{1}{2}$ " FROM CONCRETE SURFACES, UNLESS OTHERWISE NOTED.

4) FILL SPACE BETWEEN APPROACH SLAB AND WINGWALL OR ROADWAY CURB WITH ITEM 520.01. CONCRETE FILL SHALL BE A MINIMUM DEPTH OF 6 INCHES.

5) APPROACH SLABS SHALL BE PLACED AFTER THE CONCRETE DECK OVERPOUR HAS BEEN CONSTRUCTED.

6) APPROACH SLABS FOR BOTH ABUTMENTS SHALL BE CAST 2 8" BELOW FINISHED GRADE AT THE APPROACH SLAB SEATS.

Elastomeric Bearing Notes:

1) ELASTOMERIC BEARING PADS SHALL BE VIRGIN NATURAL RUBBER, HARDNESS (SHORE "A" DUROMETER) OF 60, GRADE 4.

2) ELASTOMERIC BEARINGS SHALL BE PAID AS ITEM 548.11, ELASTOMERIC BEARING PADS (F).

3) DESIGN SERVICE LOADS PER BEARING: (DESIGN METHOD A)
MAX DEAD LOAD: 16.0 KIP
MAX LIVE LOAD: 40.7 KIP

4) ANCHOR DOWELS SHALL BE GALVANIZED AFTER FABRICATION AND CONFORM TO AASHTO M232 (ASTM A153).

5) THE FABRICATOR SHALL CLEARLY MARK THE BEARING ASSEMBLIES TO ENSURE PROPER ORIENTATION IN THE FIELD.

Abutment and Wingwall Notes

1) THE CONTRACTOR SHALL BE REQUIRED TO PLACE ALL CONCRETE IN THE DRY. DEWATERING, AS REQUIRED, SHALL BE CONTINUOUS UNTIL THE ABUTMENTS AND WINGWALLS ARE BACKFILLED TO THE ELEVATION OF THE SURROUNDING WATER TABLE, UNLESS DIRECTED OTHERWISE.

2) ALL EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED ₹ EXCEPT AS NOTED.

3) CONCRETE MIXES SHALL BE AS FOLLOWS:

FOOTINGS: ITEM 520.213,

CONCRETE CLASS B, FOOTINGS (ON SOIL) (F)

ABUTMENT AND WINGWALL STEMS:

ITEM 520.12, CONCRETE CLASS A. ABOVE FOOTINGS (F)

CUEFIC WALLS AND WINGWALL CODINGS TEN 500 00

CHEEK WALLS AND WINGWALL COPINGS: ITEM 520.02,

CONCRETE CLASS AA, ABOVE FOOTINGS (F)

4) ITEM 538.2, BARRIER MEMBRANE, PEEL AND STICK — VERTICAL SURFACES (F), SHALL BE PLACED OVER THE ABUTMENT—WINGWALL VERTICAL CONSTRUCTION JOINT, 1'-0" ON EACH SIDE OF THE JOINT.

5) ITEM 534.3, WATER REPELLENT (SILANE—SILOXANE), SHALL BE APPLIED TO THE ENTIRE BRIDGE SEAT, INCLUDING THE BEARING PEDESTAL SURFACES AND ALL EXPOSED SURFACES ON BOTH ABUTMENTS AND ALL WINGWALLS TO 1'-0" BELOW THE FILL LINE.

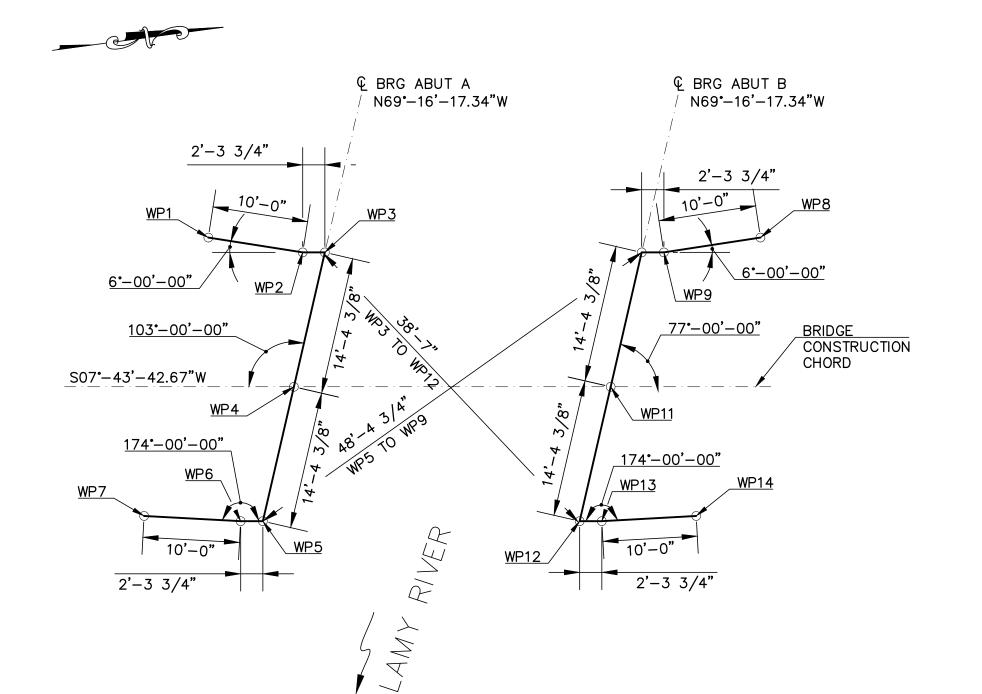
6) ALL STEEL REINFORCING IN THE ABUTMENTS AND WINGWALLS SHALL BE PAID UNDER ITEMS 544.3 — REINFORCING STEEL, (CONTRACTOR DETAILED), AND 544.31 — REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED).

7) ALL REINFORCING SHALL BE A MINIMUM OF 2 ½" FROM CONCRETE SURFACES, UNLESS NOTED OTHERWISE.

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	designed by:	LBK	drawn by:	TMA	.nd behaviore	approved by.	scale:	N/A	
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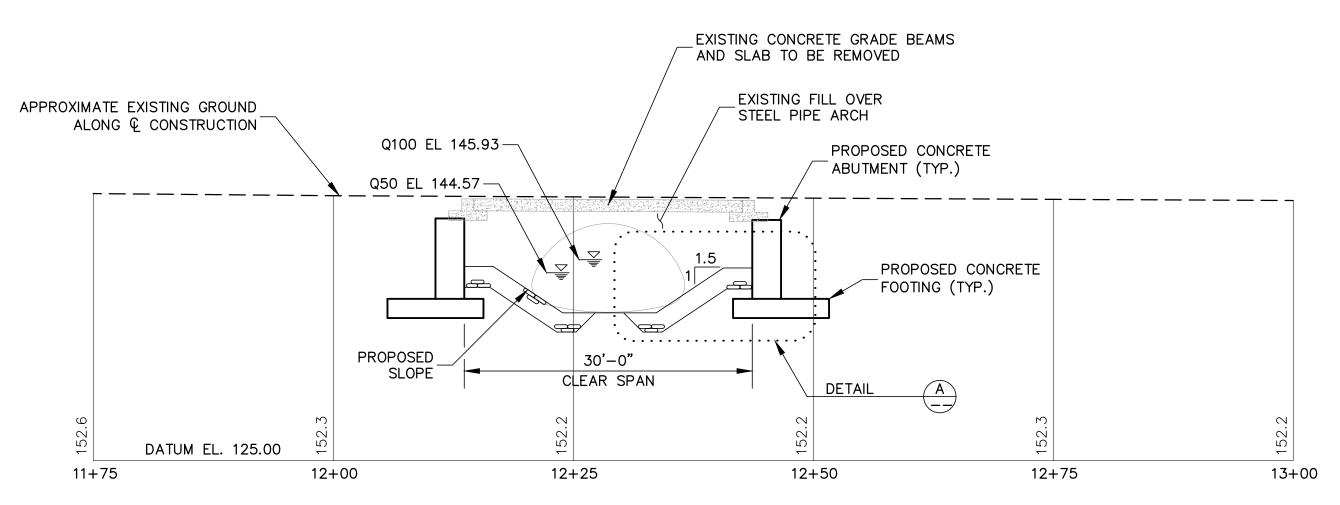
WORKIN	G POINT COC	RDINATES
WORKING POINT NO.	NORTHING	EASTING
WP1	251717.62	1168066.3
WP2	251727.34	1168068.7
WP3	251729.62	1168069.0
WP4	251724.36	1168082.9
WP5	251719.09	1168096.9
WP6	251716.80	1168096.5
WP7	251707.09	1168094.2
WP8	251774.61	1168074.1
WP9	251764.61	1168073.8
WP10	251762.32	1168073.5
WP11	251757.06	1168087.4
WP12	251751.79	1168101.3
WP13	251754.08	1168101.6
WP14	251764.07	1168101.9

Survey Layout

	LOCATION AND DESCRIPTION OF BE	ENCHMARKS		
BENCHMARK	LOCATION	NORTHING	EASTING	ELEVATION
TBM "A"	MAG. NAIL SET UP 10" IN POLE PSNH/321/40	251636.3006	1168072.4570	152.05'
TBM "B"	MAG. NAIL SET UP 8" IN POLE PSNH/321/41	251820.8404	1168099.7370	151.28'

F:\CADD\PROJECTS\1162 Madbury Nute Rd Br\Production\2—Final Design\1162 — Nute Road — BR04_Survey Layout & Channel Section.dwg Date Plotted: Jul 10, 2020 — 5:02pm Plotted By: TADAMS

HORIZONTAL DATUM: NAD83(2011), NEW HAMPSHIRE STATE PLANE COORDINATE SYSTEM VERTICAL DATUM: NAVD88 (GEOID12A)



Proposed Channel Section

Scale: 1"=10'

ITEM NO	SUMMARY OF BRIDGE QUANTITIES ITEM DESCRIPTION	QUANTITY	UNIT
	GRANULAR BACKFILL (BRIDGE) (F)		
209.201	, , , , , , , , , , , , , , , , , , ,	210	CY
502	REMOVAL OF EXISTING BRIDGE STRUCTURE	1	U
503.1	WATER DIVERSION STRUCTURES ROCK BRIDGE EXCAVATION	1 5	U C Y
504.2	STRUC TURAL FILL	40	CY
			CY
520.02	CONCRETE CLASS AA, ABOVE FOOTINGS (F)	10	
520.03	CONCRETE CLASS AA, APPROACH SLABS (F)	40	CY
520.12	CONCRETE CLASS A, ABOVE FOOTINGS (F)	85	CY
520.213	CONCRETE CLASS B, FOOTINGS (ON SOIL) (F)	60	СҮ
520.7	CONCRETE BRIDGE DECK (QC/QA) (F)	20	СҮ
528.312	PRESTRESSED CONCRETE BRIDGE DECK, BUTTED DECK BEAMS (F)	970	SF
534.3	WATER REPELLENT (SILANE/SILOXANE)	15	GAL
538.2	BARRIER MEMBRANE, PEEL AND STICK — VERTICAL SURFACES (F)	10	SY
538.5	BARRIER MEMBRANE, HEAT WELDED (F)	150	SY
541.4	PVC WATERSTOPS, NH TYPE 4 (F)	45	LF
544.3	REINFORCING STEEL (CONTRACTOR DETAILED)	11400	LB
544.31	REINFORCING STEEL, EPOXY COATED (CONTRACTOR DETAILED)	7850	LB
544.7	SYNTHETIC FIBER REINFORCEMENT (F)	280	LB
548.11	ELASTOMERIC BEARING PADS (F)	14	EA
559.41	ASPHALTIC PLUG FOR CRACK CONTROL (F)	55	LF
562.1	SILICONE JOINT SEALANT (F)	60	LF
585.2	STONE FILL, CLASS B	210	СҮ
593.411	GEOTEXTILE; PERM CONTROL CL.1, NON-WOVEN	430	SY

	designed by: LBK	drawn by:	TMA	approved by:		ıe: D <i>TED</i>
	date: July 2020	project no:	1162	checked by:	1	scale: AS NOTED
	Town of Madbury, New Hampshire		Nitte Road		Bellamy River Crossing Replacement	Survey Layout & Channel Section
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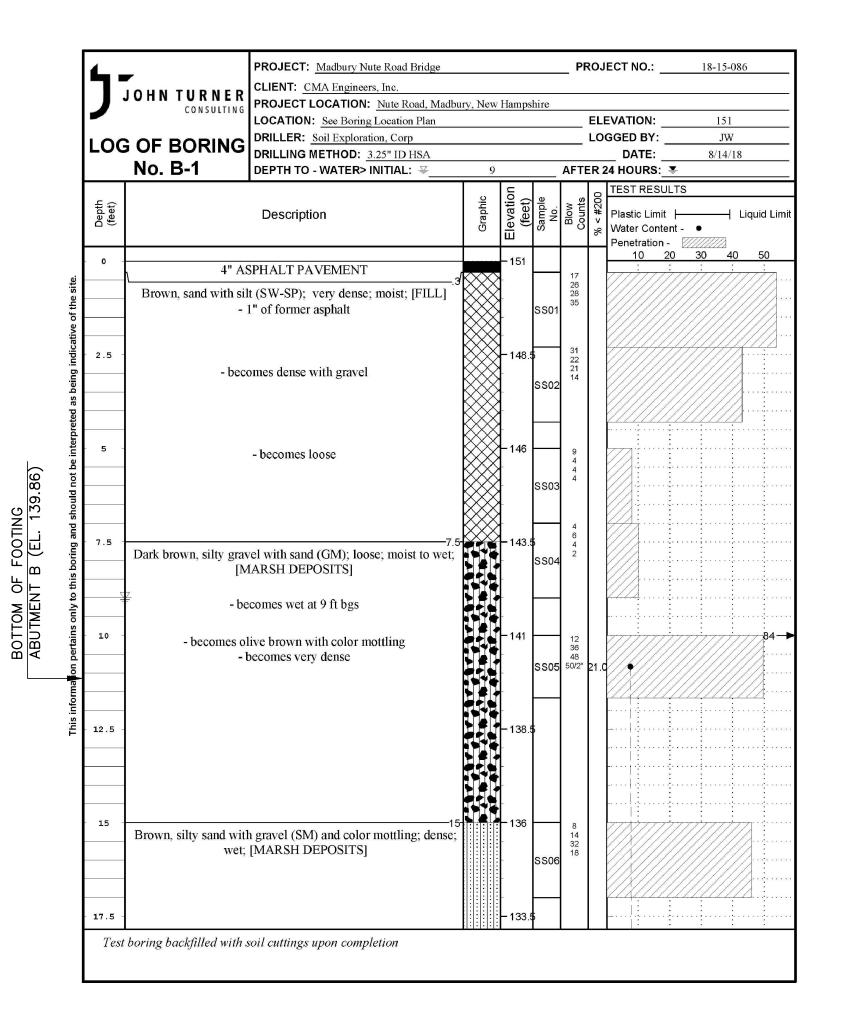
PROPOSED STREAM BED 3'-6" 1.5 EL 140.4 ITEM 583.3 2'-0" THICK 1 2'-0"
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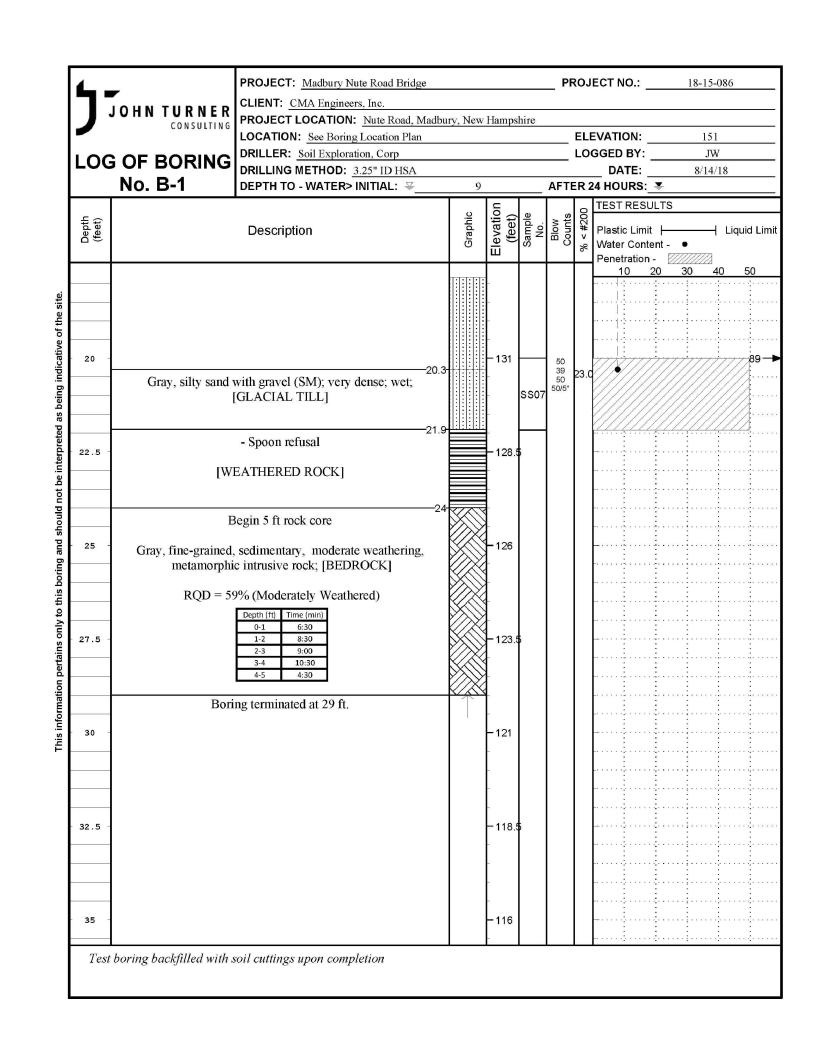
Detail	A
Scale: 1/4"=1'-0"	

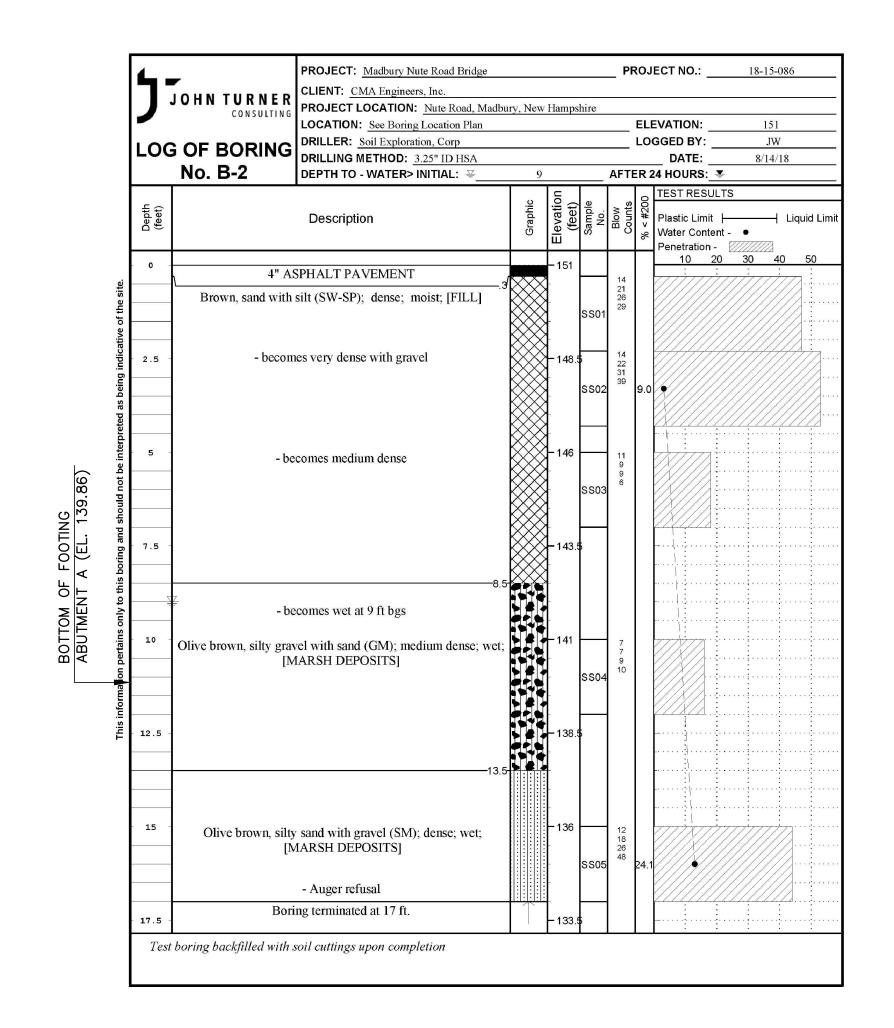
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Station 12+65, 6.4 Left

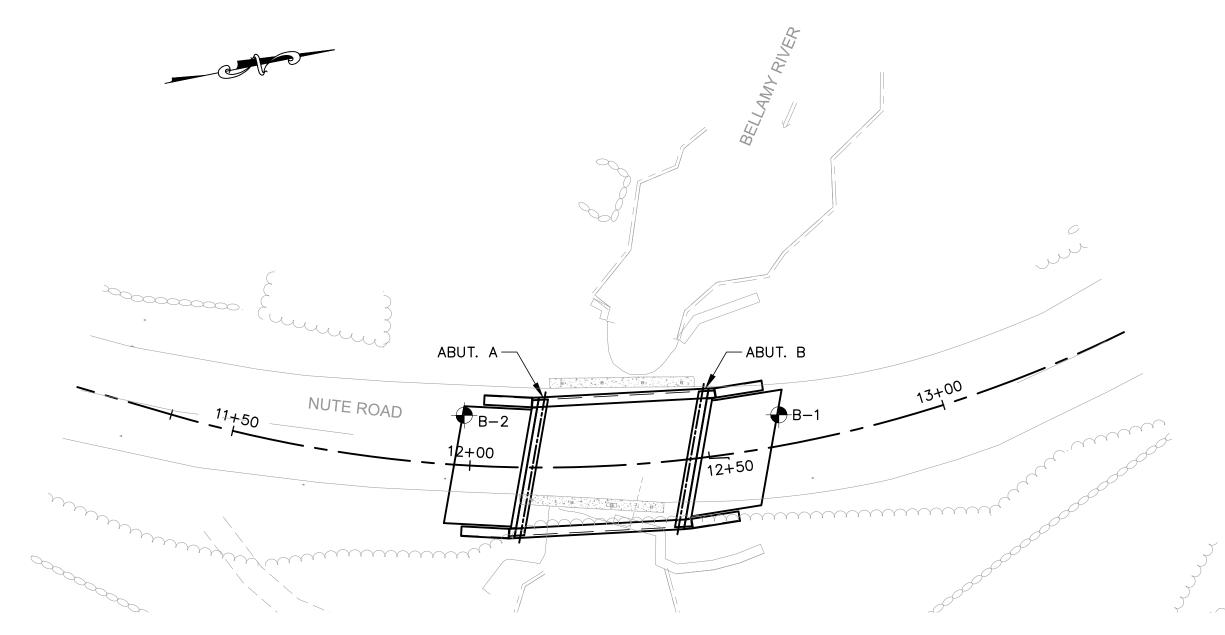
Boring No. B-1

Boring No. B-2
Station 11+98, 10.4 Left









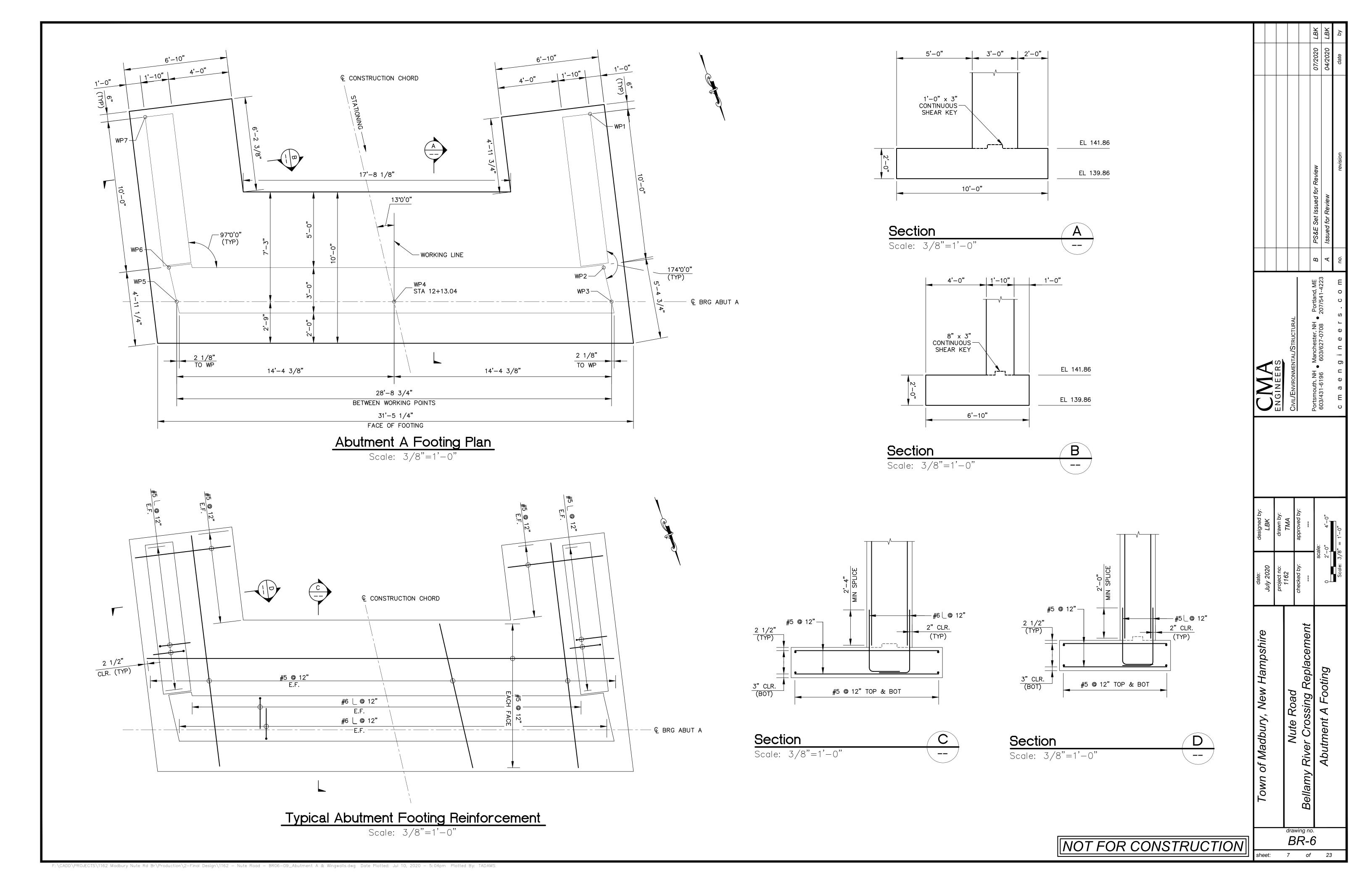
Boring Location Plan

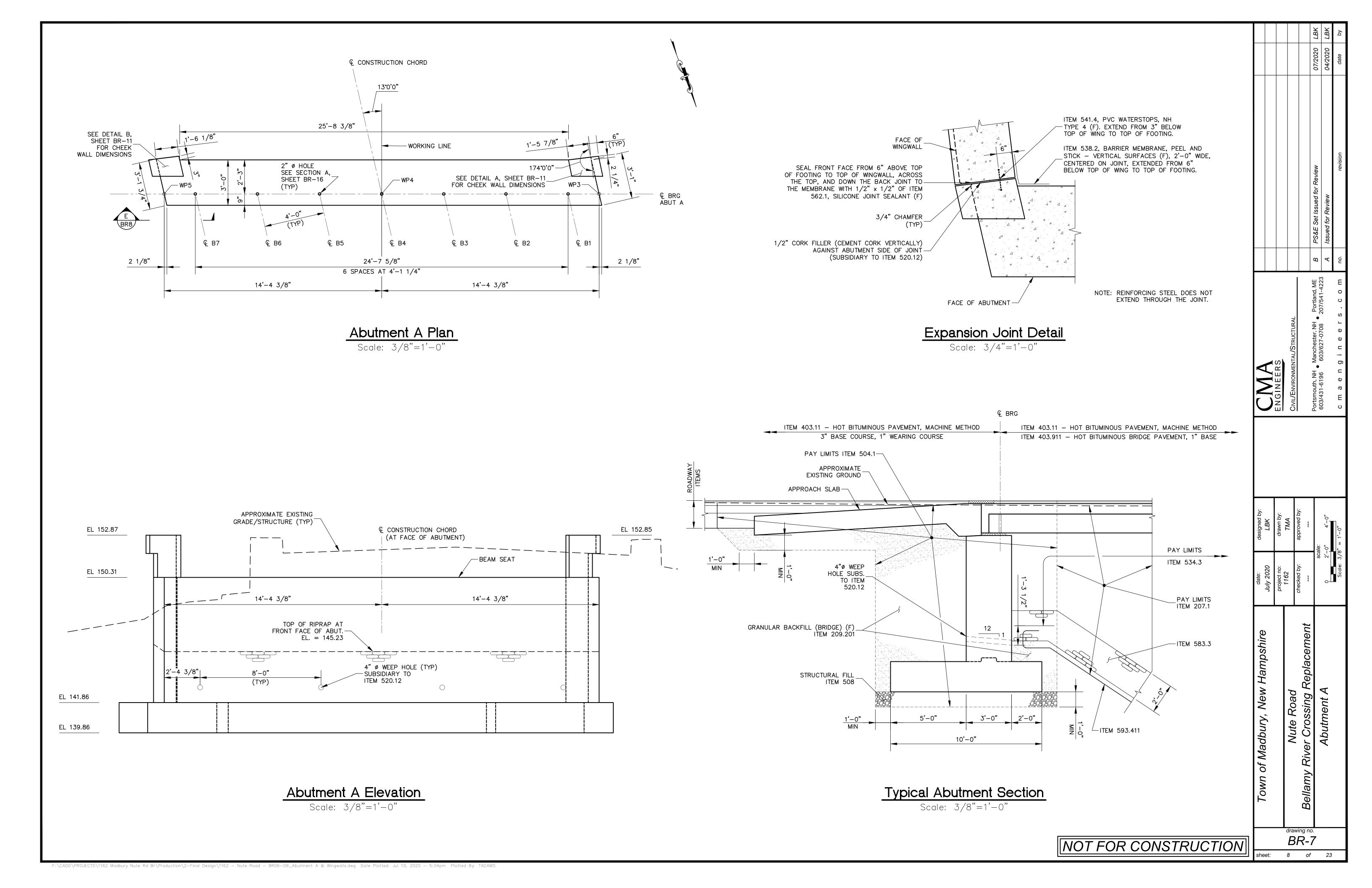
Not to Scale

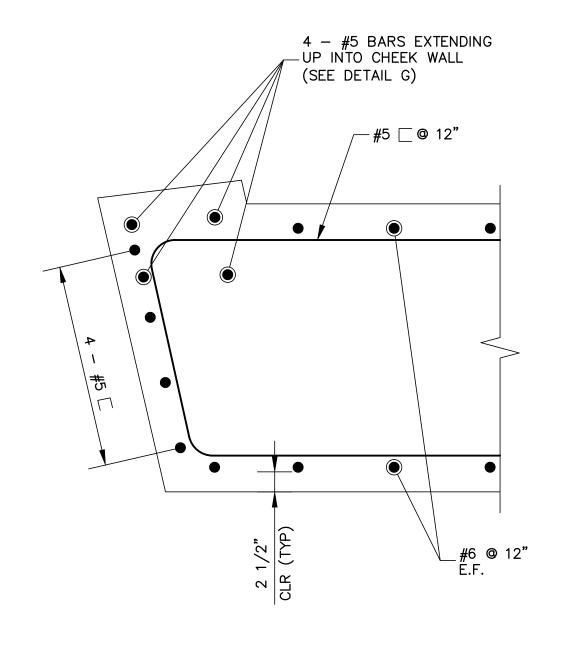
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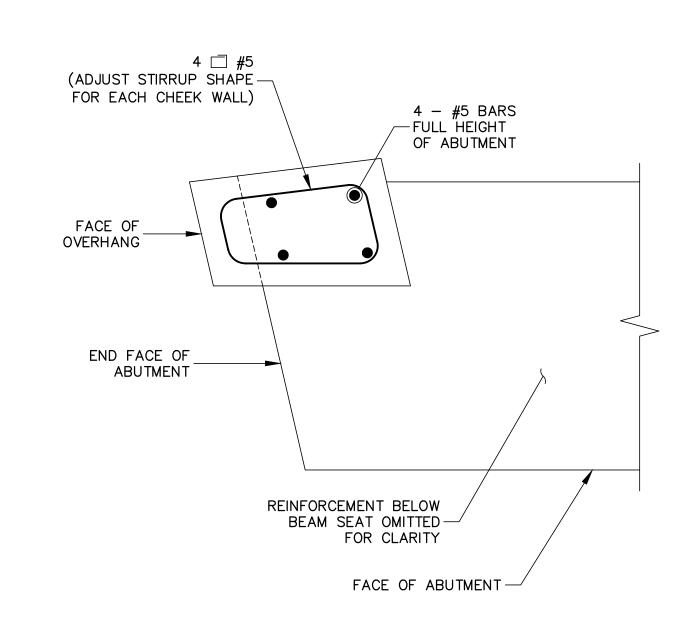
Town of Madbury, New Hampshire July 2020 LBK project no: drawn by: 1162 TMA Checked by: approved by:
date: July 2020 project no: 1162 checked by:
Town of Madbury, New Hampshire Nute Road Bellamy River Crossing Replacement Borings

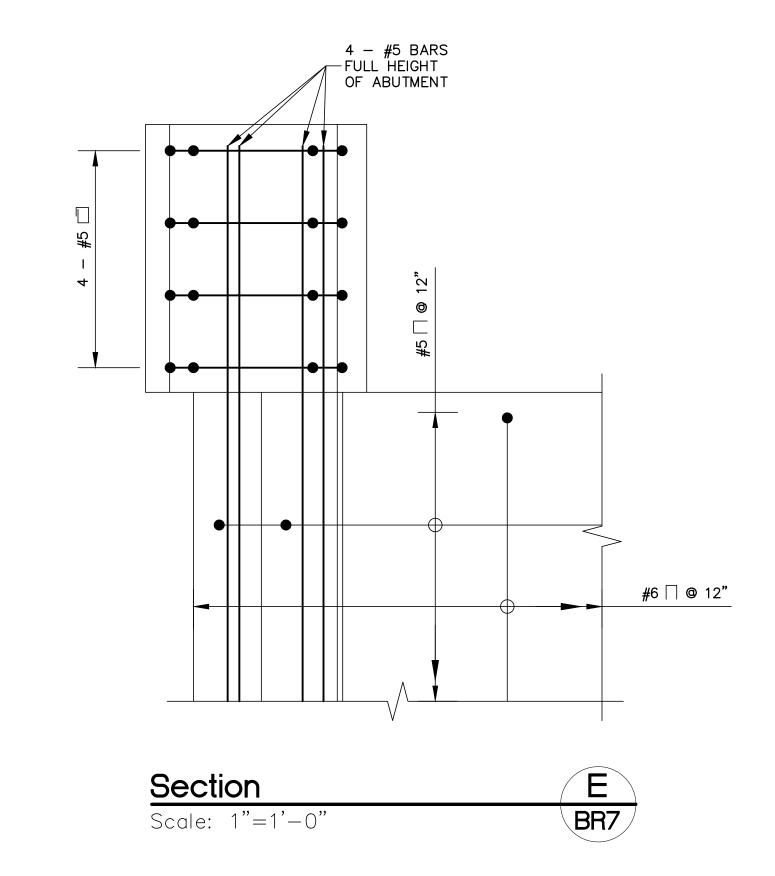
Portland, ME 207/541-4223

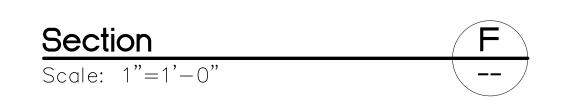


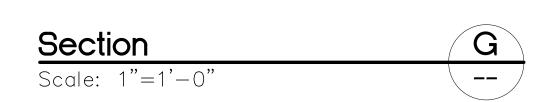


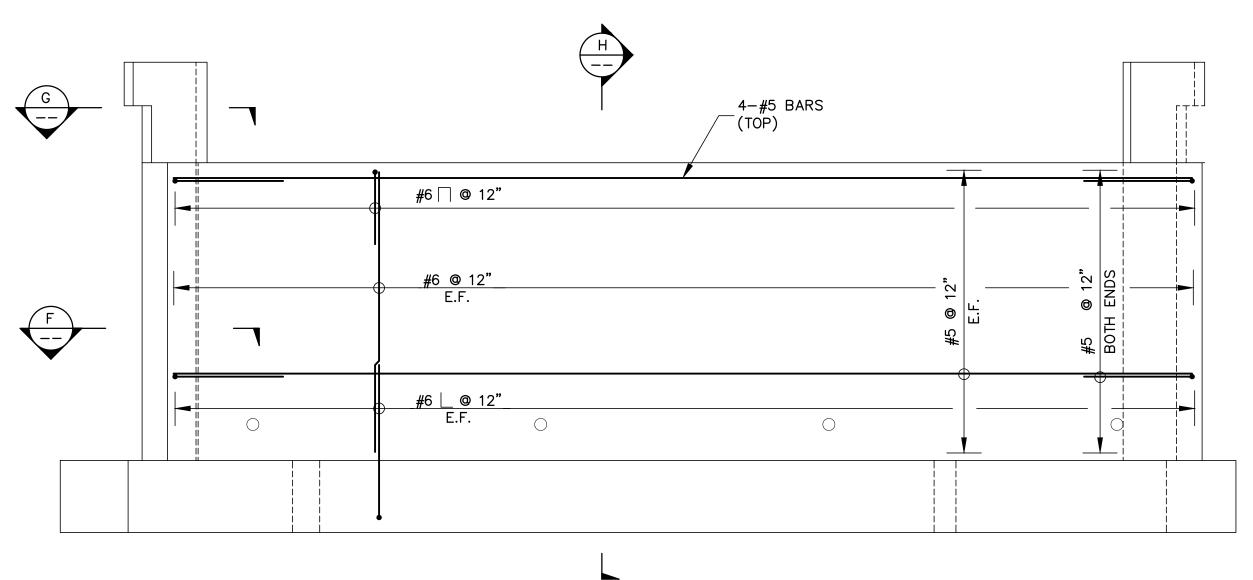






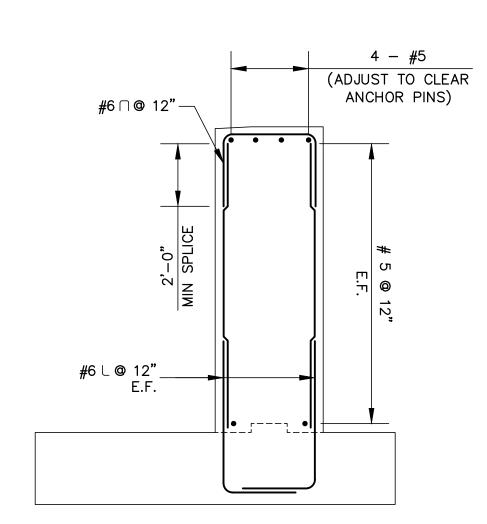






Typical Abutment Reinforcement

Scale: 3/8"=1'-0"



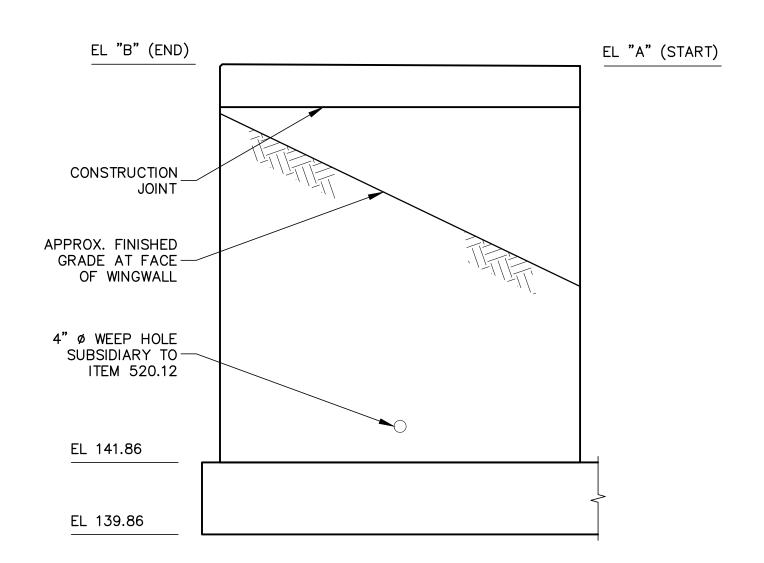
Section
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designed by:	LBK	drawn by:	TMA	approved by:	i	Scale:	2'-0" 4'-0"	Scale: $3/8$ " = 1'-0"
date:	July 2020	project no:	1162	checked by:		os	0 2,	Scale: 3/
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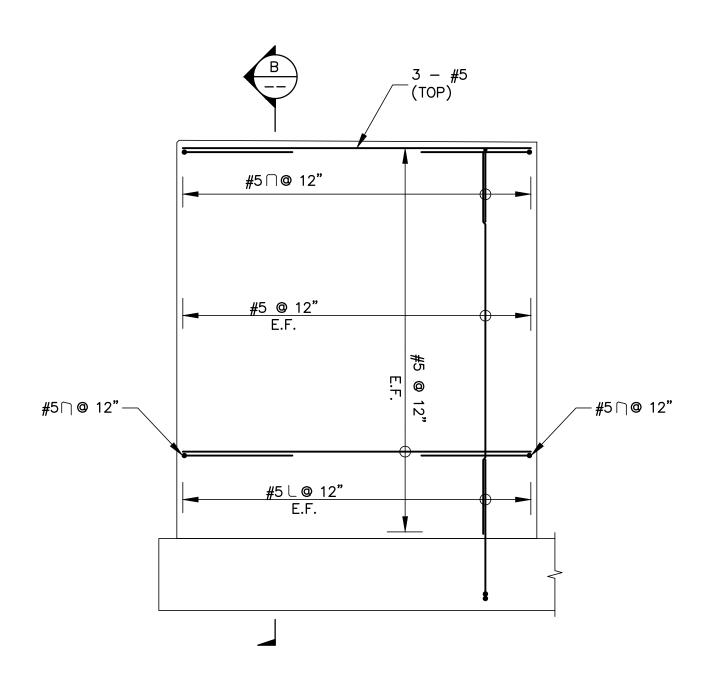


Typical Wingwall Elevation Scale: 3/8"=1'-0"

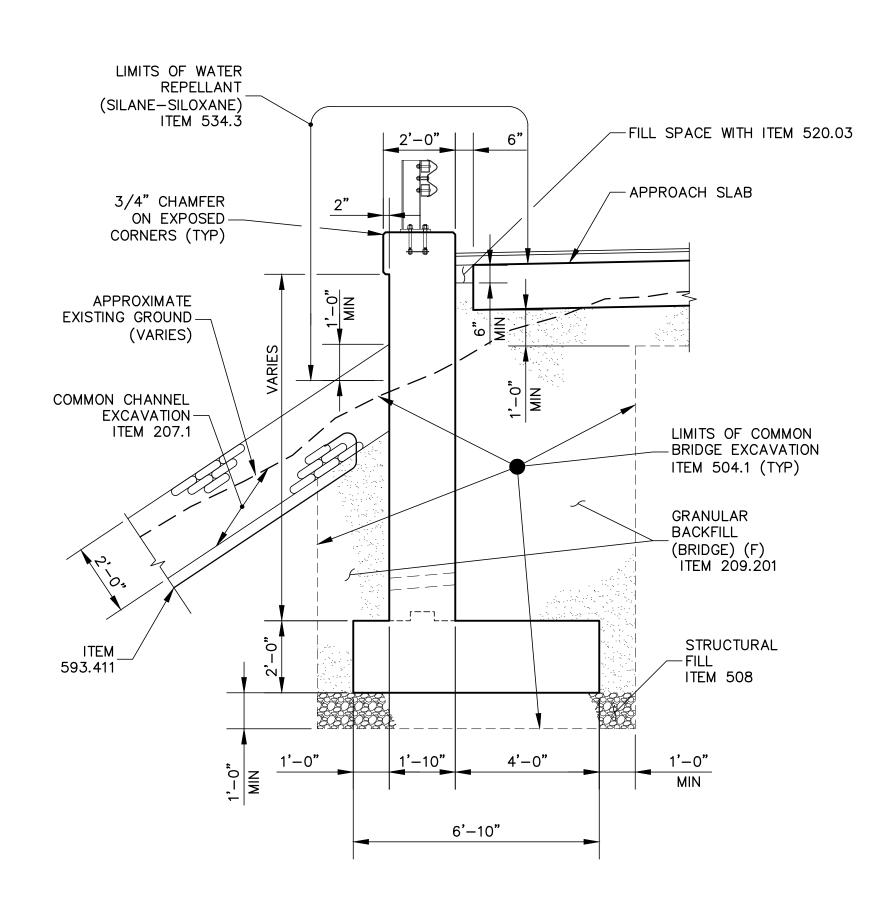
WIN	IGWALL ELEVATIO	NS
WINGWALL	EL. "A"	EL. "B"
SOUTHWEST	152.86	152.91
SOUTHEAST	152.88	152.93
NORTHWEST	152.67	152.62
NORTHEAST	152.71	152.66

NOTE: "START" OF WINGWALL IS THE END ADJACENT TO THE ABUTMENT

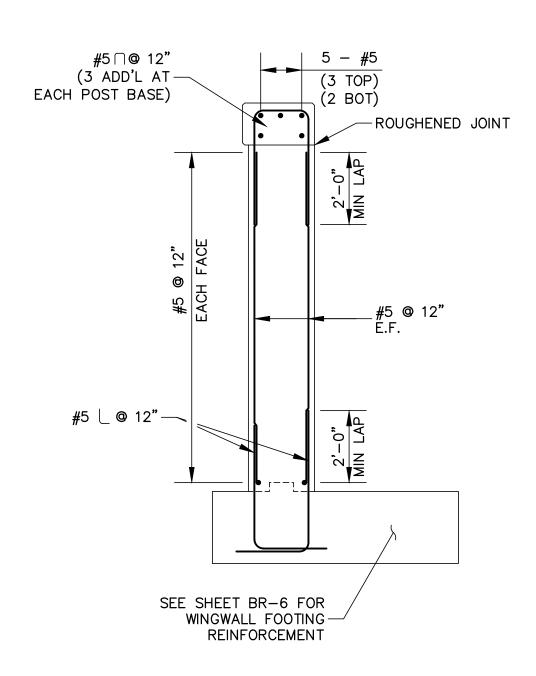
F:\CADD\PROJECTS\1162 Madbury Nute Rd Br\Production\2-Final Design\1162 - Nute Road - BR06-09_Abutment A & Wingwalls.dwg Date Plotted: Jul 10, 2020 - 5:04pm Plotted By: TADAMS



Typical Wingwall Reinforcement Scale: 3/8"=1'-0"

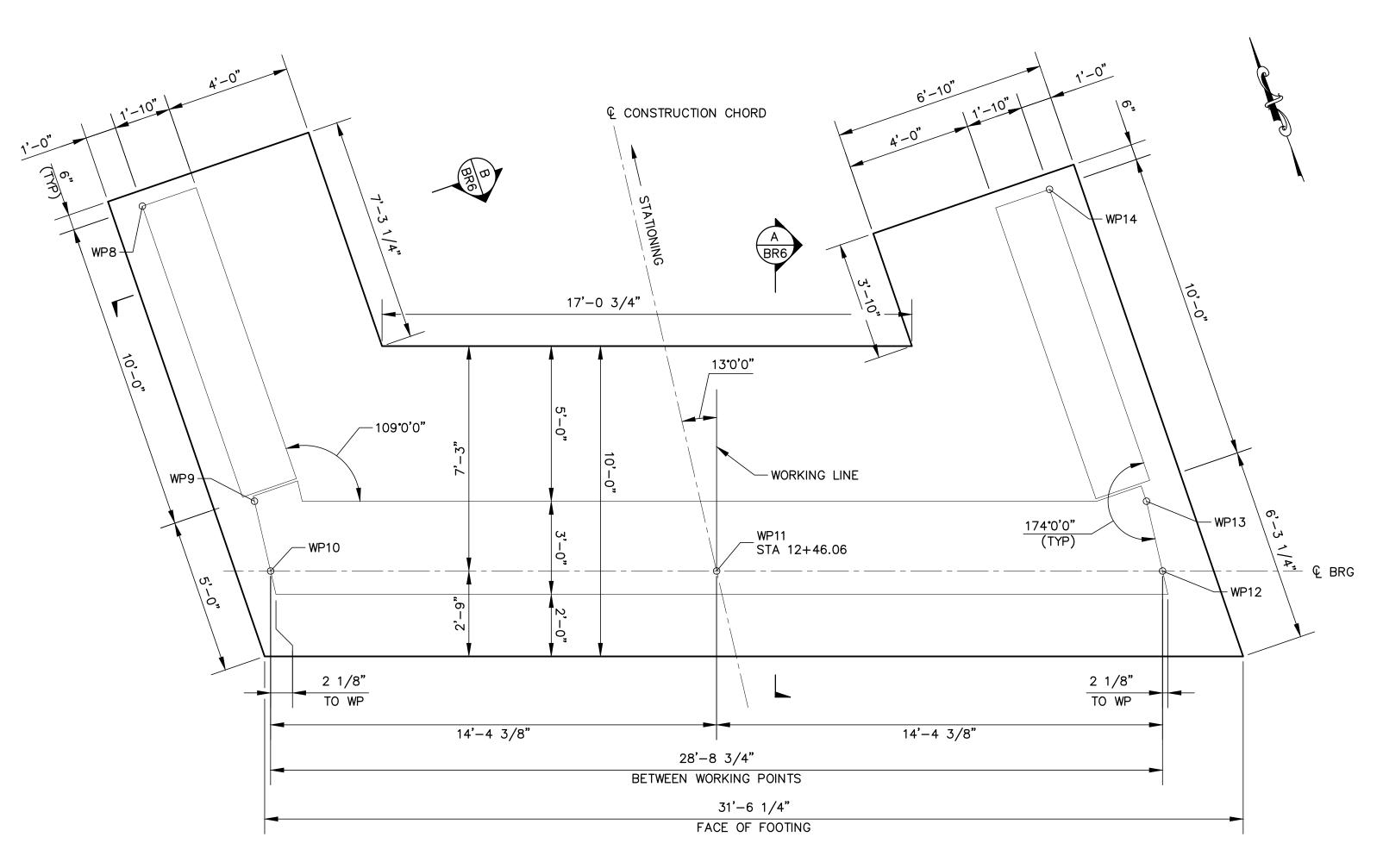


Typical Wingwall Section Scale: 3/8"=1'-0"

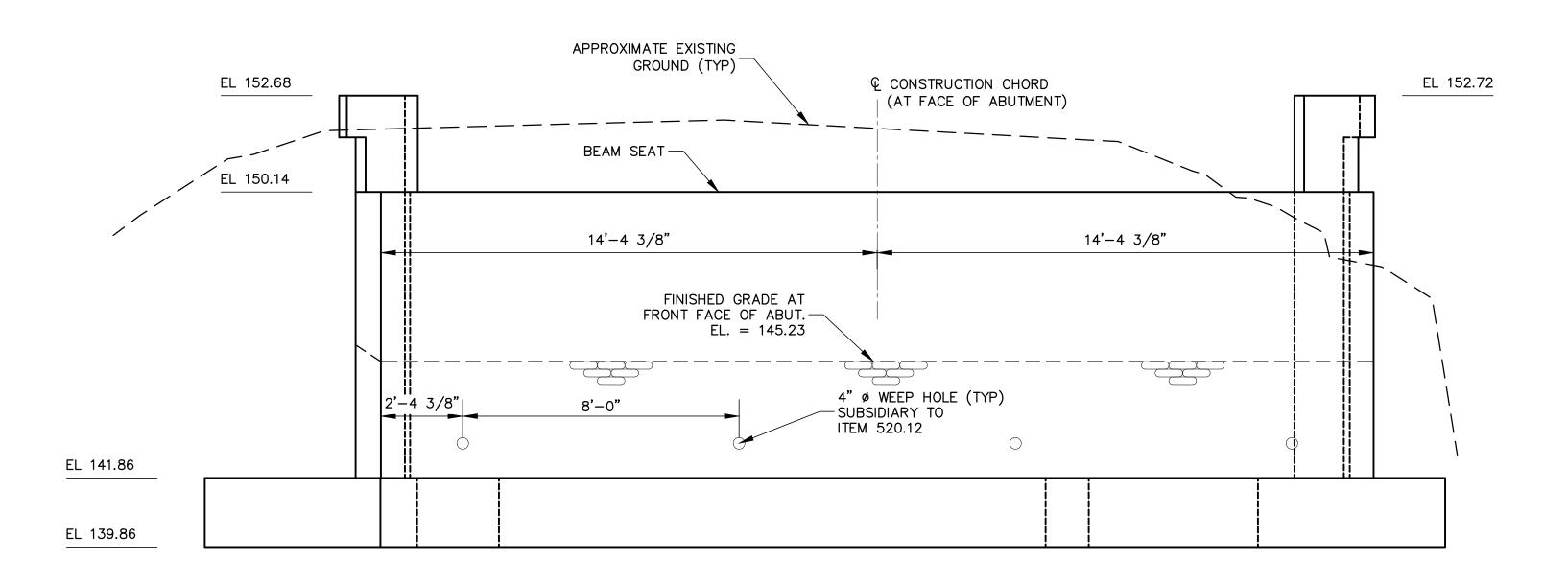


Sect	ion	B
Scale:	3/8"=1'-0"	

designed by: LBK	drawn by: TMA	approved by: 	4'-0"
$\vdash \vdash$	project no: dra 1162 7	checked by: appr	scale: 0 2'-0" 4 Scale: 3/8" = 1'-0"
Town of Madbury, New Hampshire	Nithe Road	Bellamy River Crossing Replacement	Abutment A Wingwalls
		wing no. BR-9	
sheet:	10	of	23



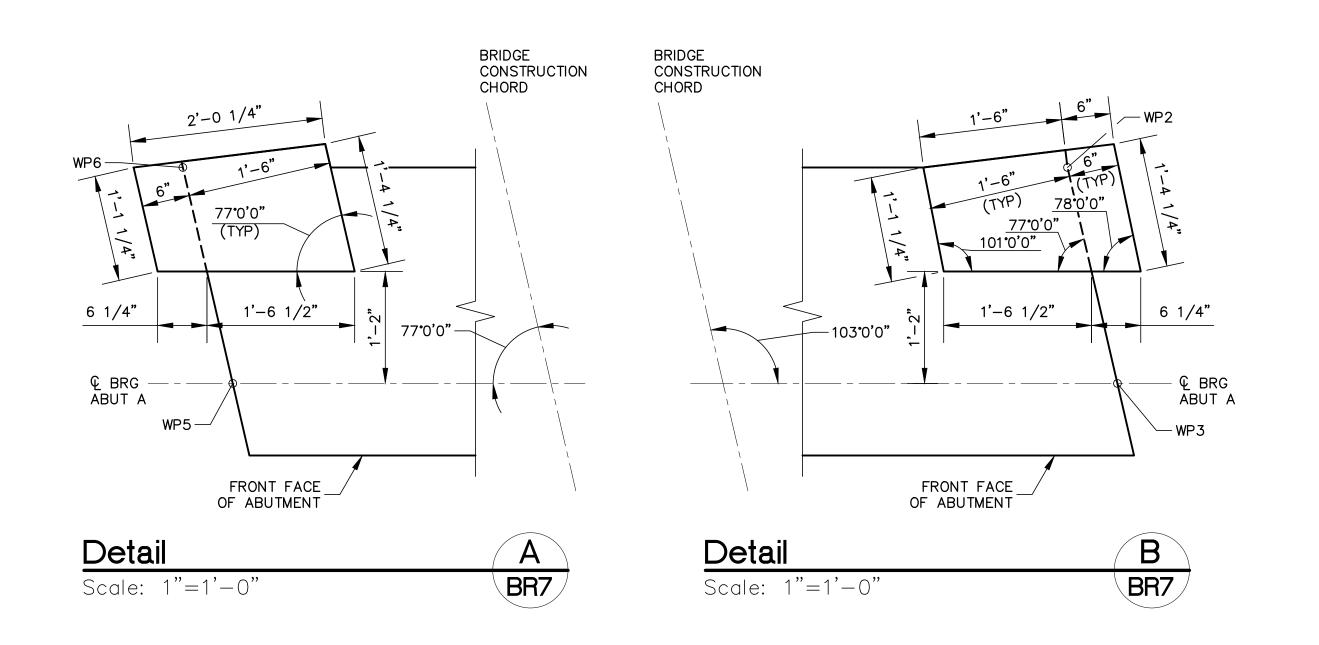
Abutment B Footing Plan Scale: 3/8"=1'-0"

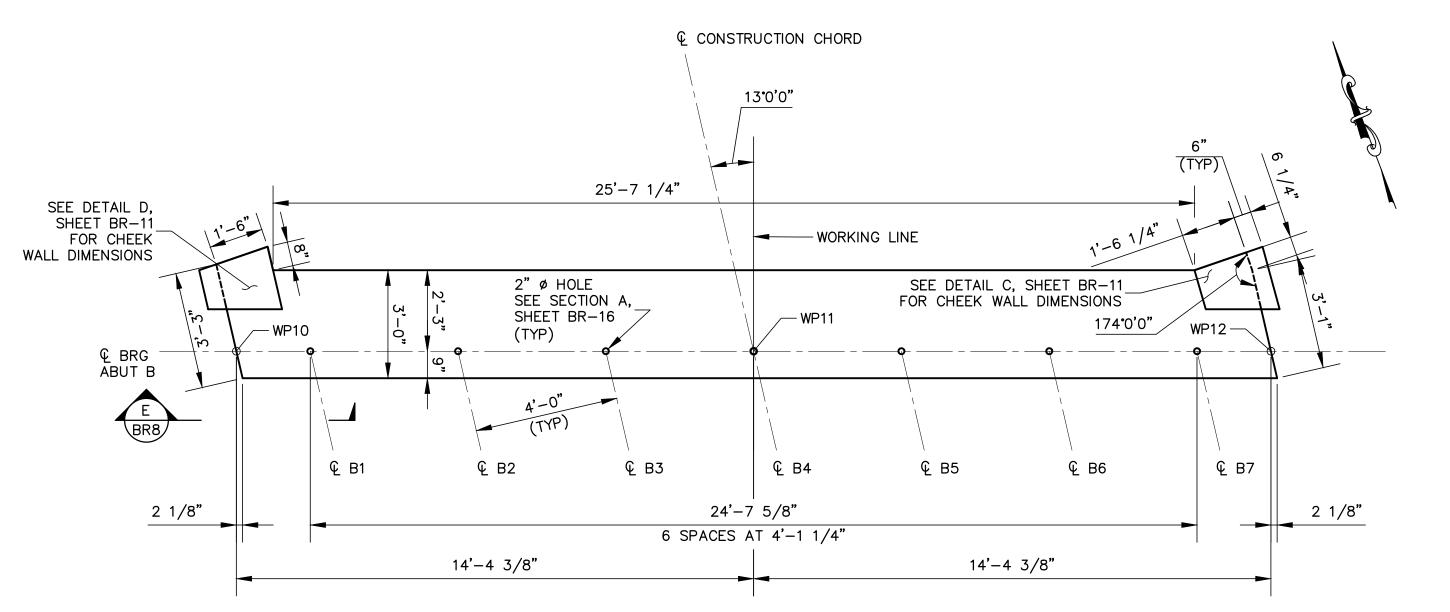


Abutment B Elevation Scale: 3/8"=1'-0"

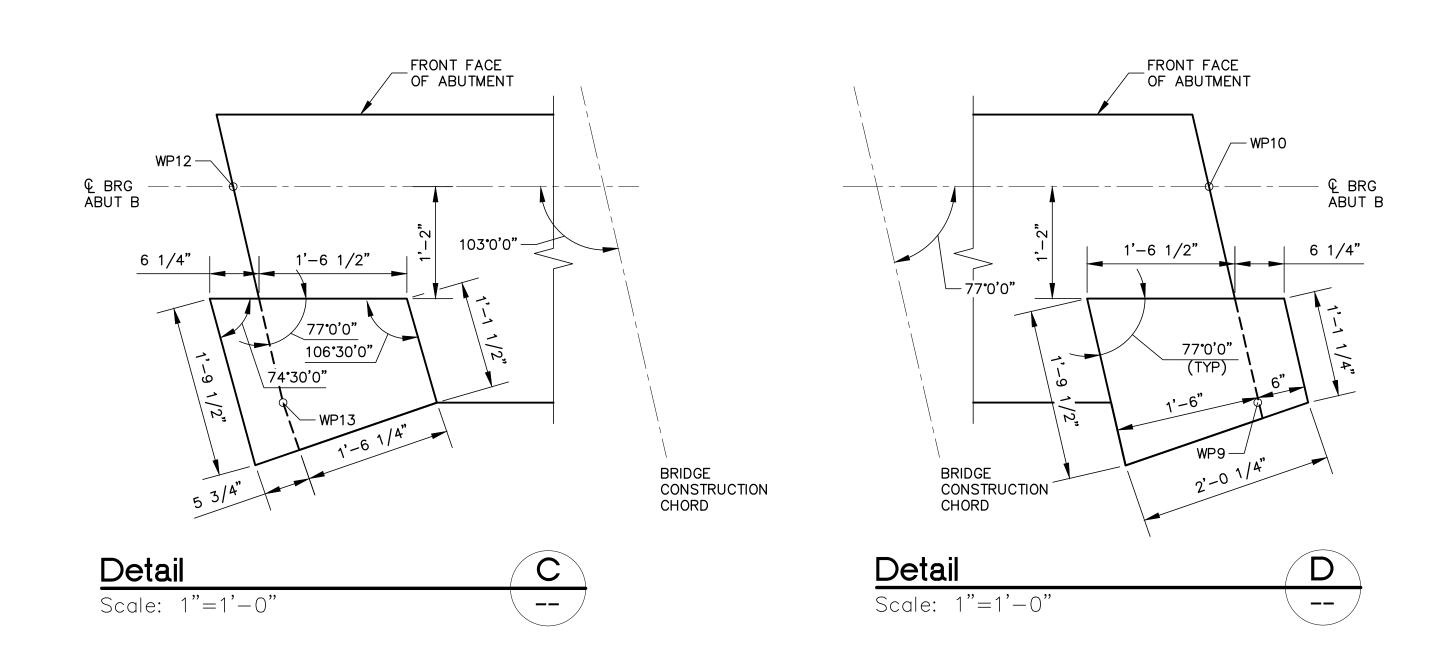
F:\CADD\PROJECTS\1162 Madbury Nute Rd Br\Production\2-Final Design\1162 - Nute Road - BR10-13_Abutment B & Wingwalls.dwg Date Plotted: Jul 10, 2020 - 5:05pm Plotted By: TADAMS

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	:designed by	LBK	drawn by:	TMA	approved by:		scale:	2'-0" 4'-0"	Scale: $3/8$ " = 1'-0"
	date:	July 2020	project no:	1162	checked by:		S	0	Scale: 3,
	Town of Madbury, New Hampshire			Nitte Road		Bellamv River Crossing Replacement		Abutment B Footing	
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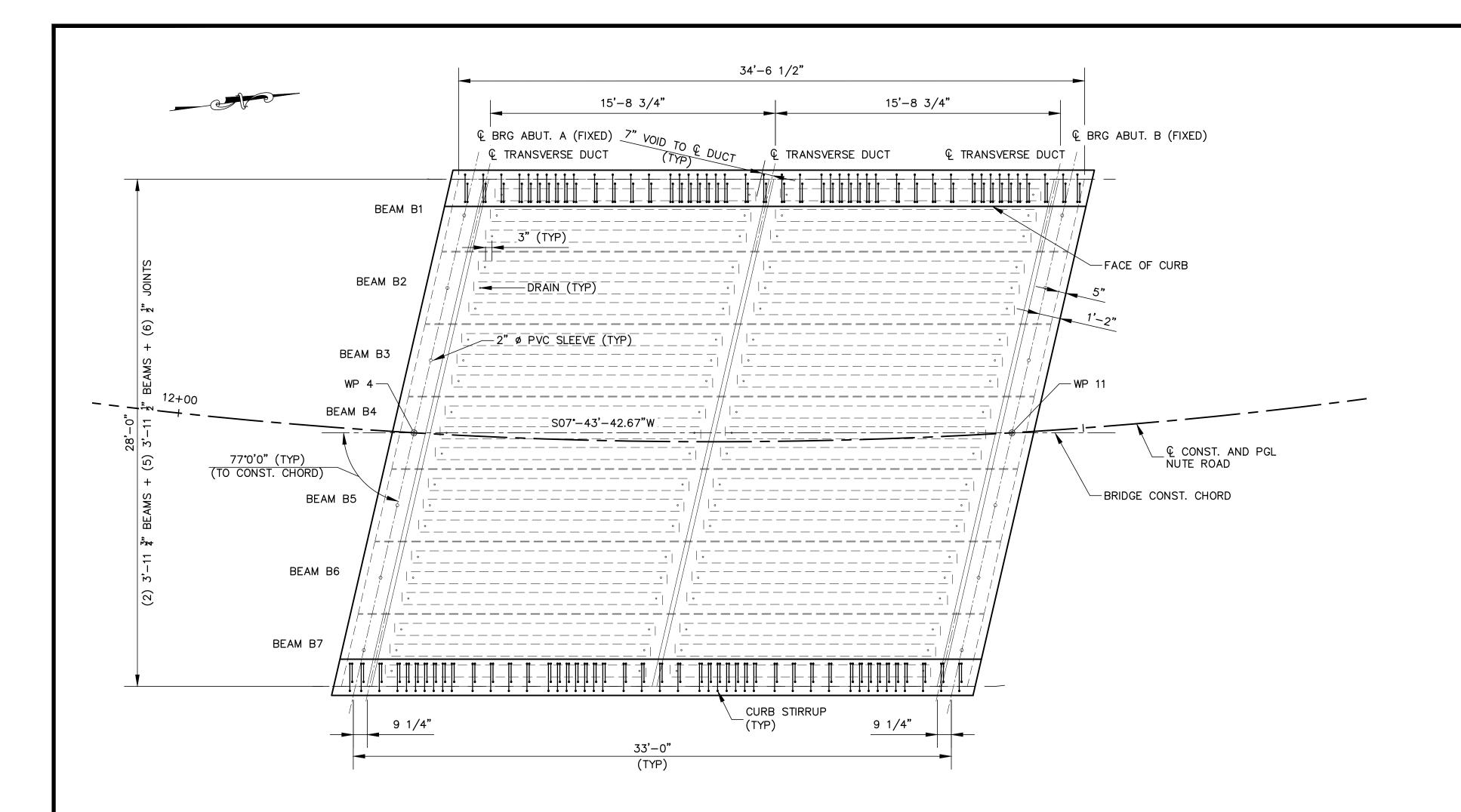




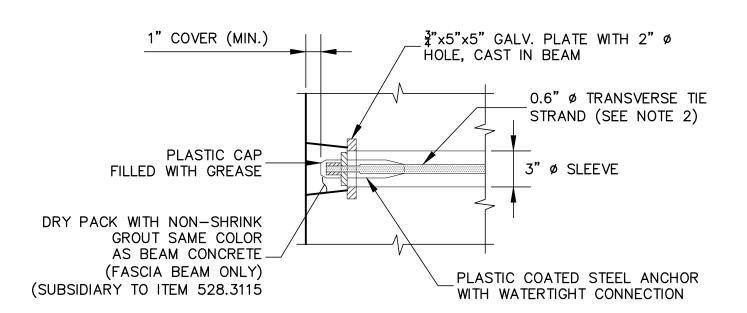
Abutment B Plan Scale: 3/8"=1'-0"



Town of Madbury, New Hampshire July 2020 LBK project no: drawn by: 1162 TMA Nute Road Checked by: approved by: L Scale: Abutment B Scale: 3/8" = 1'-0"	CMA	ENGINEERS	CIVIL/ENVIRONMENTAL/STRUCTURAL	AM Lacter of LIN retendent LIN discourse	603/431-6196 603/627-0708 207/541-4223	c m a e n g i n e e r s . c o m
Town of Madbury, New Hampshire Town of Madbury, New Hampshire July 3 11 11 11 11 11 11 11	designed by: LBK	drawn by: TMA	approved by:		-,4	$3^{\circ} = 1^{\circ} - 0^{\circ}$
Town drawing no.	date: July 2020	project no: 1162	checked by:		sc. 0 2'-	Scale: 3/8
					Abutment B	
sheet: 12 of 23		E	BR.			



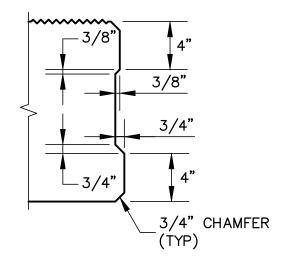
Framing Plan Scale: 1/4"=1'-0"



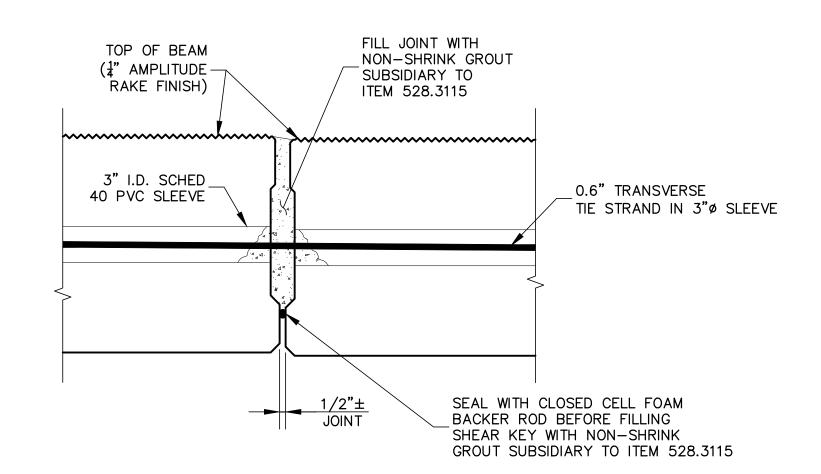
TRANSVERSE TIE NOTES:

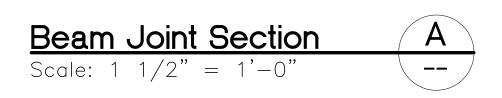
- 1. OTHER ANCHORAGE SYSTEMS MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. ALTERNATE ANCHORAGE SYSTEMS SHALL BE WATERTIGHT AND CORROSION PROOF.
- 2. TRANSVERSE TIES SHALL BE COVERED BY A SEAMLESS POLYPROPYLENE SHEATH (WITH CORROSION INHIBITING GREASE BETWEEN THE STRAND AND SHEATH) FOR THE FULL LENGTH OF THE STRAND, EXCEPT AT THE ANCHORAGE LOCATION.

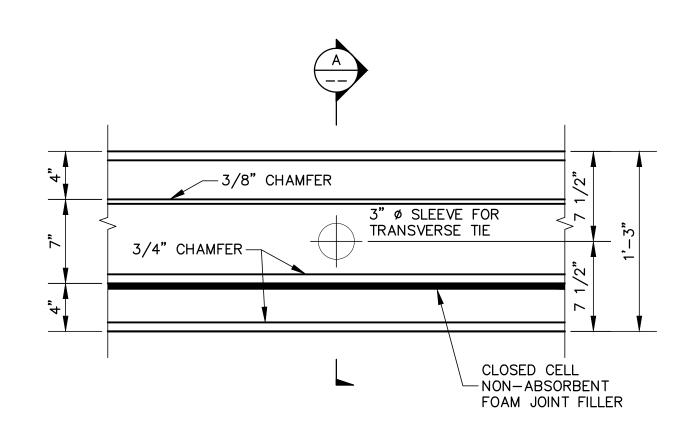
Transverse Tie Anchorage Scale: 1 1/2"=1'-0"



Scale: 1 1/2"=1'-0"





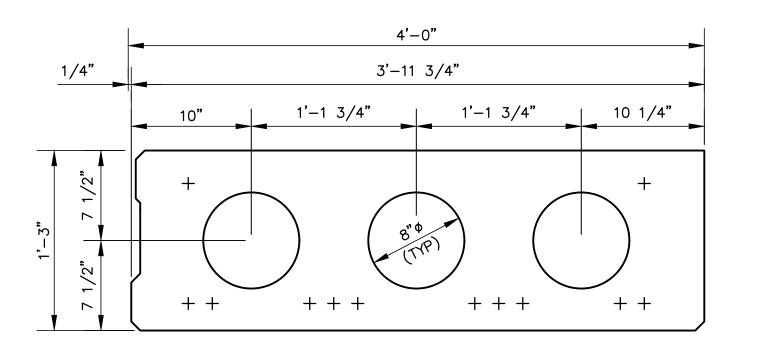


TRANSVERSE TIES ARE LOCATED AT THE MID POINT AND AT BOTH ENDS OF THE DECK BEAMS

Typical Beam Elevation at Transverse Tie Locations

> Scale: 1 1/2"=1NOT FOR CONSTRUCTION

			07/2020	date
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MA	ENGINEERS Civil /Envidonmental /Stbilctlibal	1	603/431-6196 603/627-0708 207/541-4223	c m a e n g i n e e r s . c o m
designed by:	drawn by:	approved by:	8, 603	
date: July 2020	project no: 1162	checked by: 	scale:	Scale: $1/4$ " = $1'-0$ "
Fown of Madbury, New Hampshire	Nute Road	ellamy Rive	Prestressed Beam Layout	
70		Be		J
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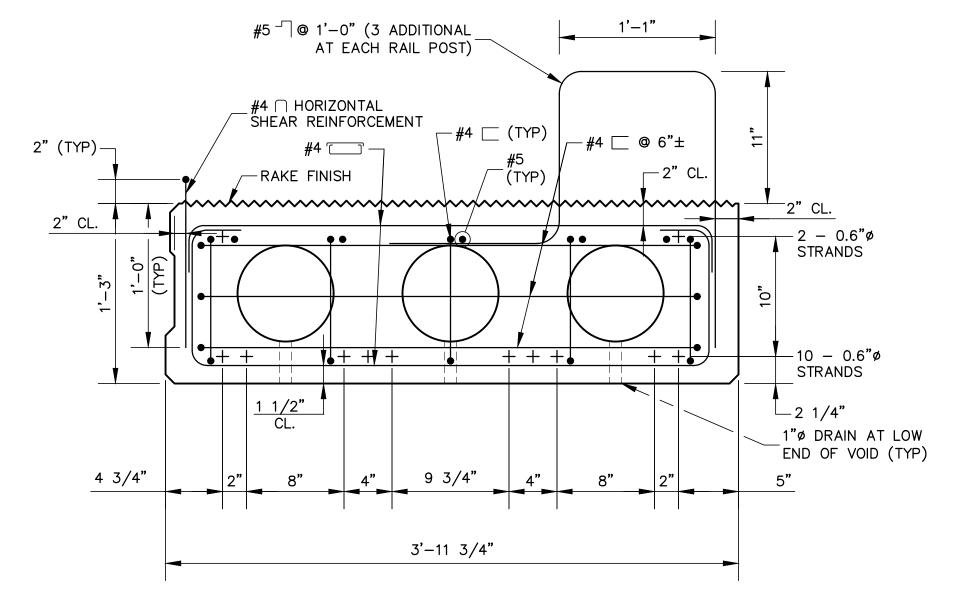


NOTES:

- 1. + DENOTES STRAIGHT STRANDS
- 2. SEE SHEAR KEY DETAIL ON SHEET BR-12
- 3. SEE END OF BEAM PLAN FOR STIRRUP SPACING

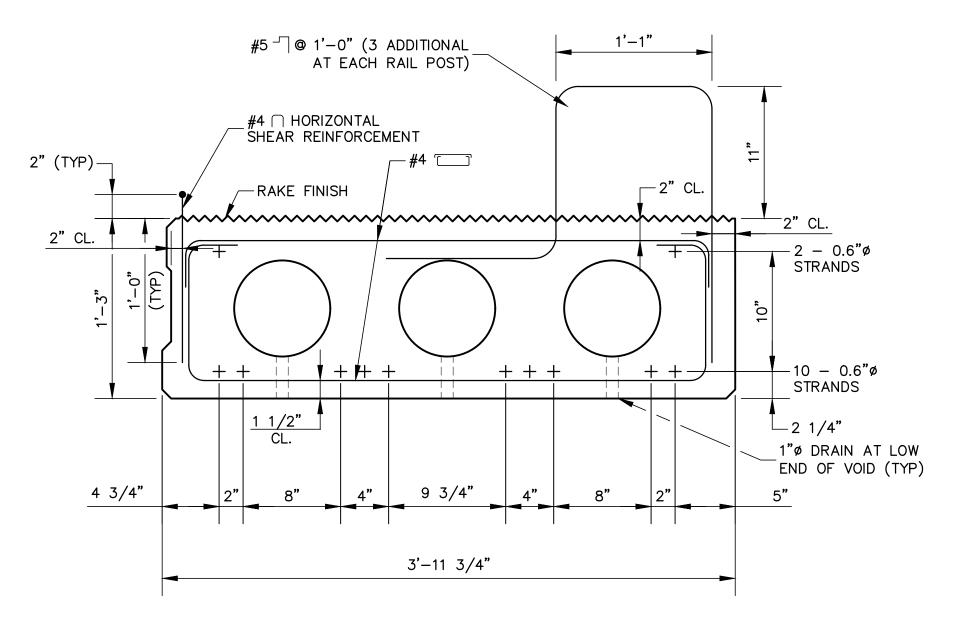
B1 and B7 Typical Section

Scale: 1 1/2"=1'-0"



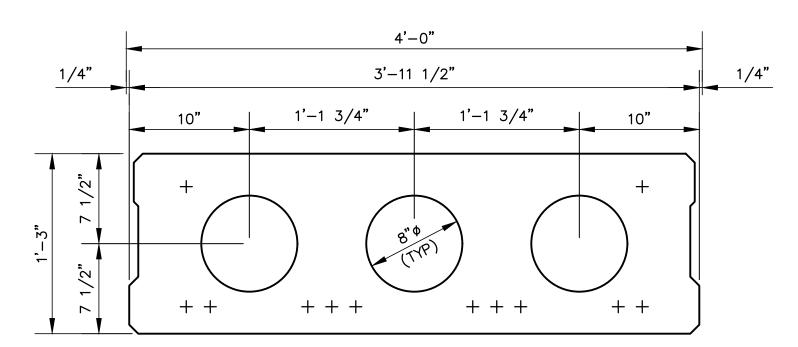
B1 and B7 End Section

Scale: 1 1/2"=1'-0"



B1 and B7 Midspan Section

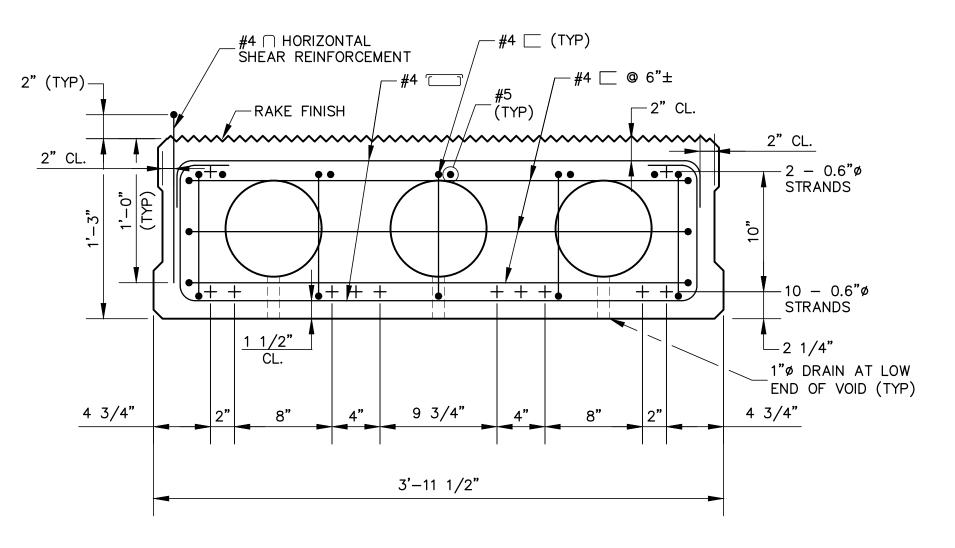
Scale: 1 1/2"=1'-0"



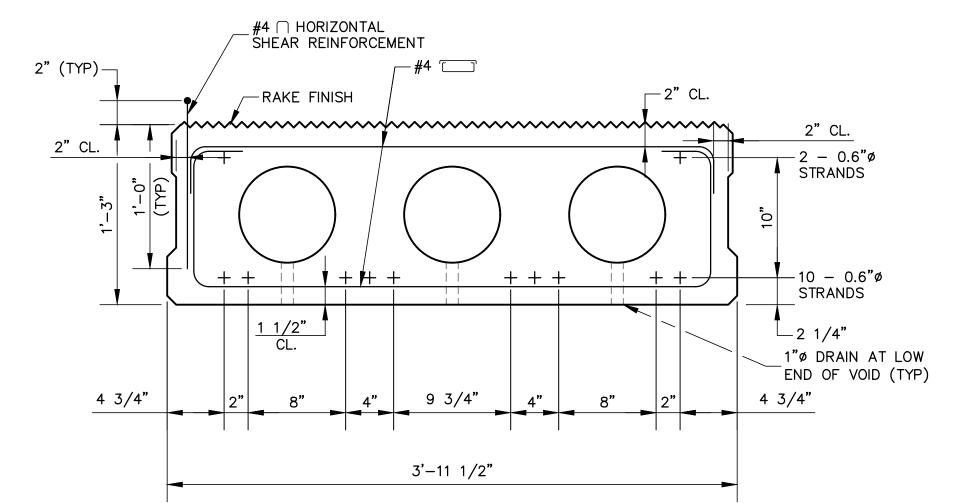
NOTES:

- 1. + DENOTES STRAIGHT STRANDS
- 2. SEE SHEAR KEY DETAIL ON SHEET BR-12
- 3. SEE END OF BEAM PLAN FOR STIRRUP SPACING

B2-B6 Typical Section Scale: 1 1/2"=1'-0"



B2-B6 End Section



B2-B6 Midspan Section Scale: 1 1/2"=1'-0"

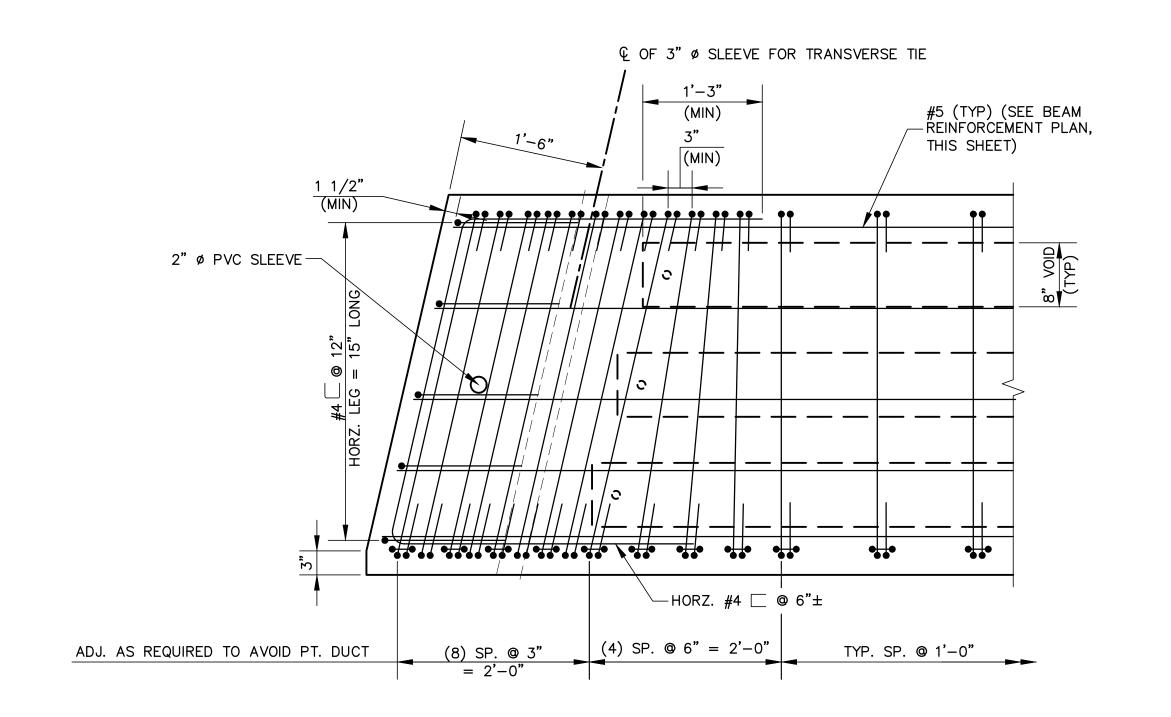
	CAMBER AT MID-SPAN (IN)										
	PRESTRESS	SELF WEIGHT	DEAD LOAD	SUPERIMPOSED DL	TOTAL AT ERECTION	TOTAL AT FINAL					
BEAM B1	1.476	-0.598	-0.114	-0.038	0.724	0.641					
BEAM B2	1.476	-0.598	-0.136	-0.038	0.704	0.577					
BEAM B3	1.476	-0.598	-0.157	-0.038	0.683	0.511					
BEAM B4	1.476	-0.598	-0.175	-0.020	0.683	0.514					
BEAM B5	1.476	-0.598	-0.162	-0.038	0.678	0.496					
BEAM B6	1.476	-0.598	-0.142	-0.038	0.698	0.556					
BEAM B7	1.476	-0.598	-0.120	-0.038	0.720	0.623					

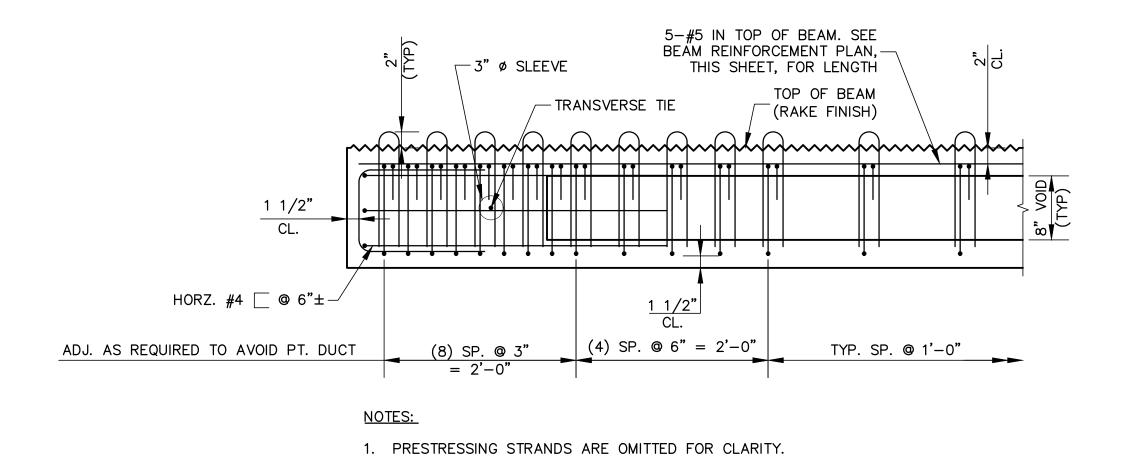
NOTE: NEGATIVE VALUES INDICATE DOWNWARD DEFLECTION.

	1					ACING OF CO			
LOCATION				INCR	REASING STAT	IONS ——			
LOCATION	Ф BEARING	1/8 PT	1/4 PT	3/8 PT	1/2 PT	5/8 PT	3/4 PT	7/8 PT	€ BEARING
LEFT CURBLINE	152.06	152.04	152.02	152.00	151.98	151.96	151.93	151.91	151.89
PROFILE GRADE LINE	152.32	152.31	152.30	152.28	152.27	152.24	152.22	152.19	152.16
RIGHT CURBLINE	152.08	152.07	152.06	152.04	152.03	152.00	151.98	151.95	151.93

TOP OF DECK POUR ELEVATION = F.G. - PAVEMENT THICKNESS + Δ SLAB + Δ SUPERIMPOSED DL

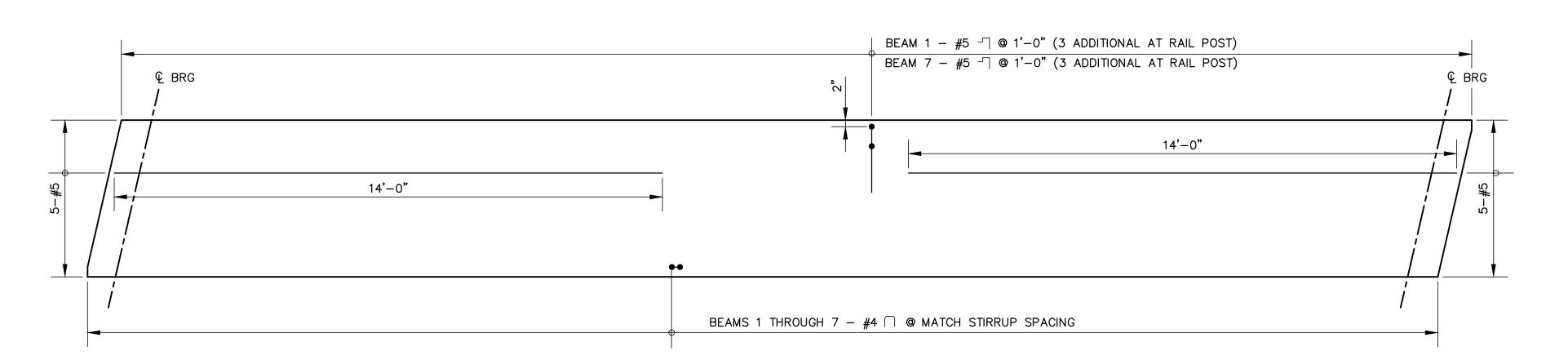
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End of Beam Plan
Scale: 1"=1'-0"

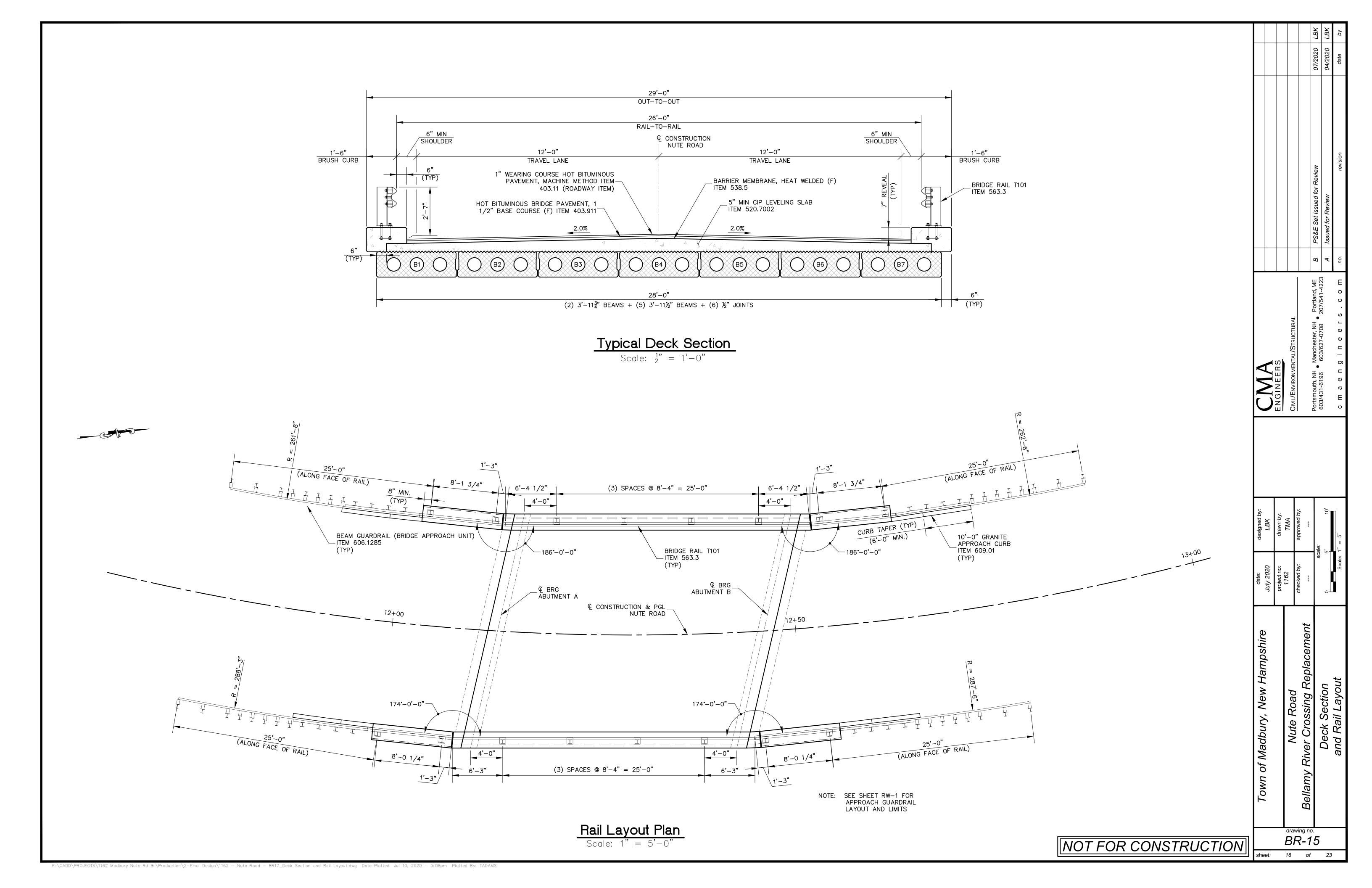
End of Beam Elevation

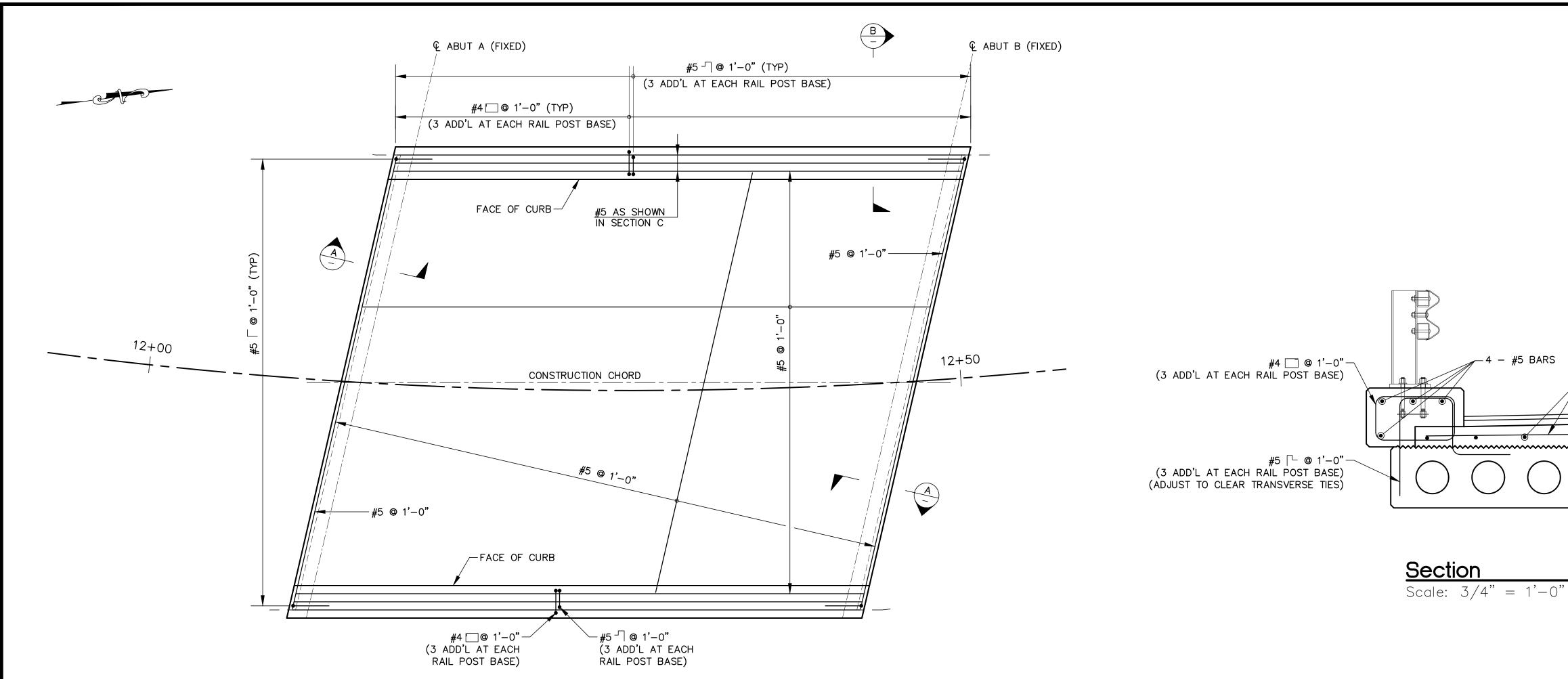


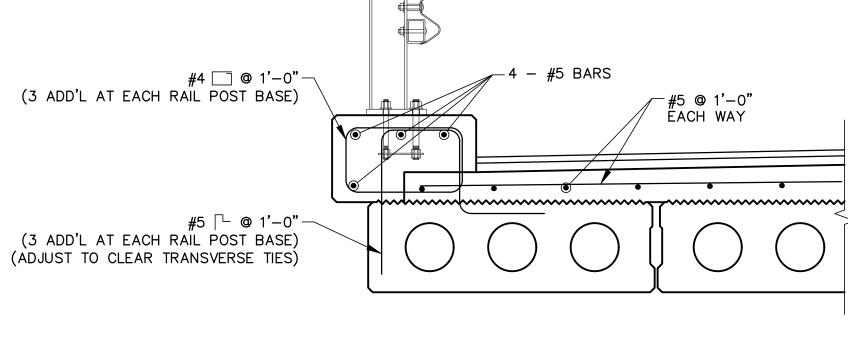
Beam Reinforcing Plan Scale: 1/2"=1'-0"

drawing no.

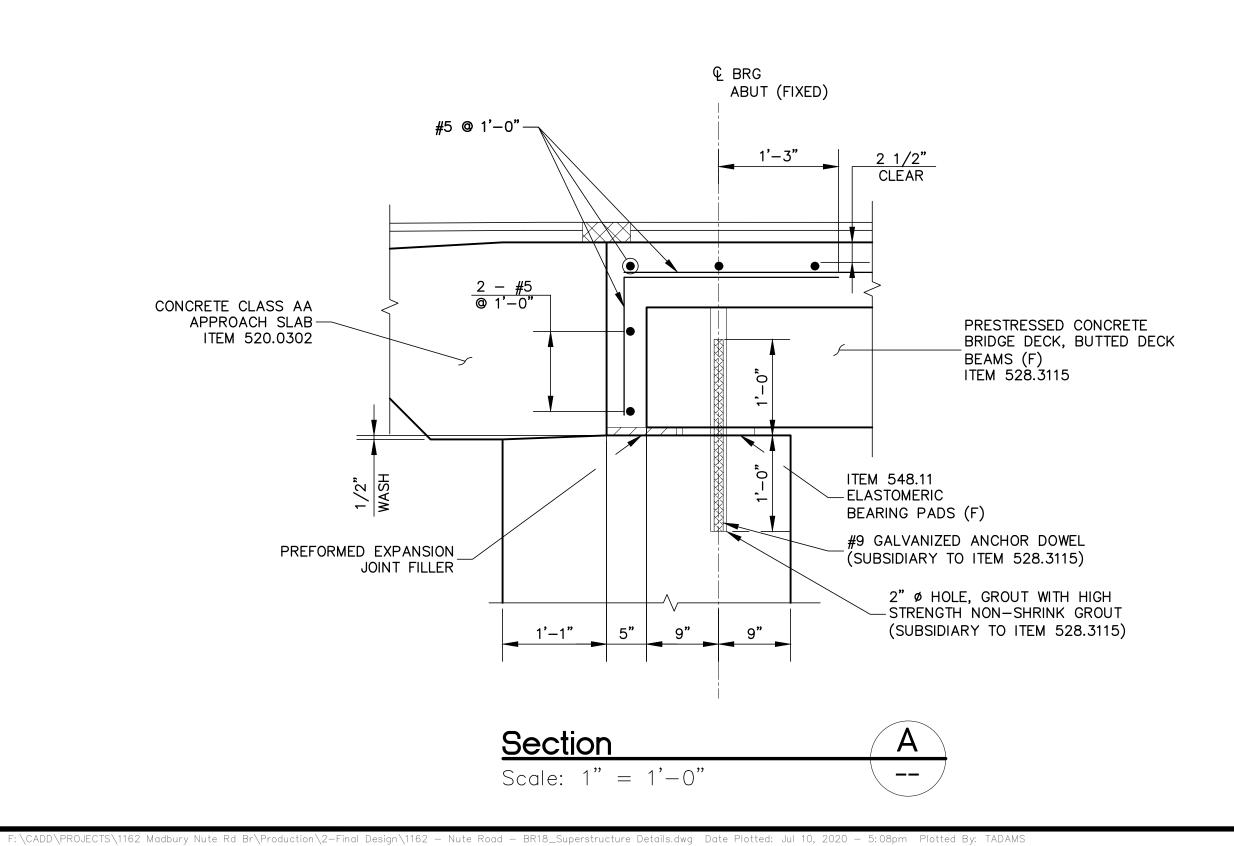
BR-14 sheet: 15 of 23

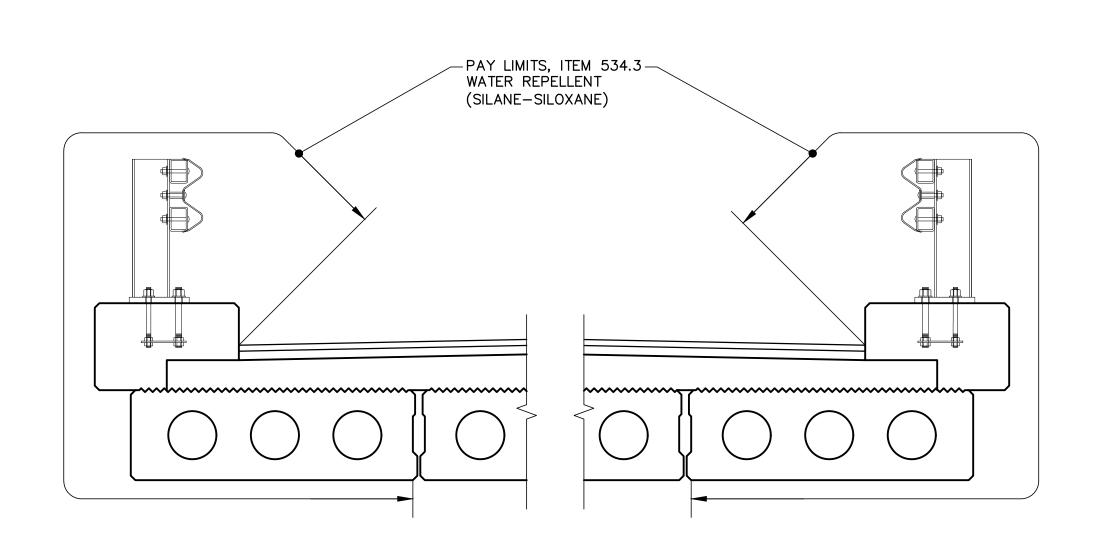






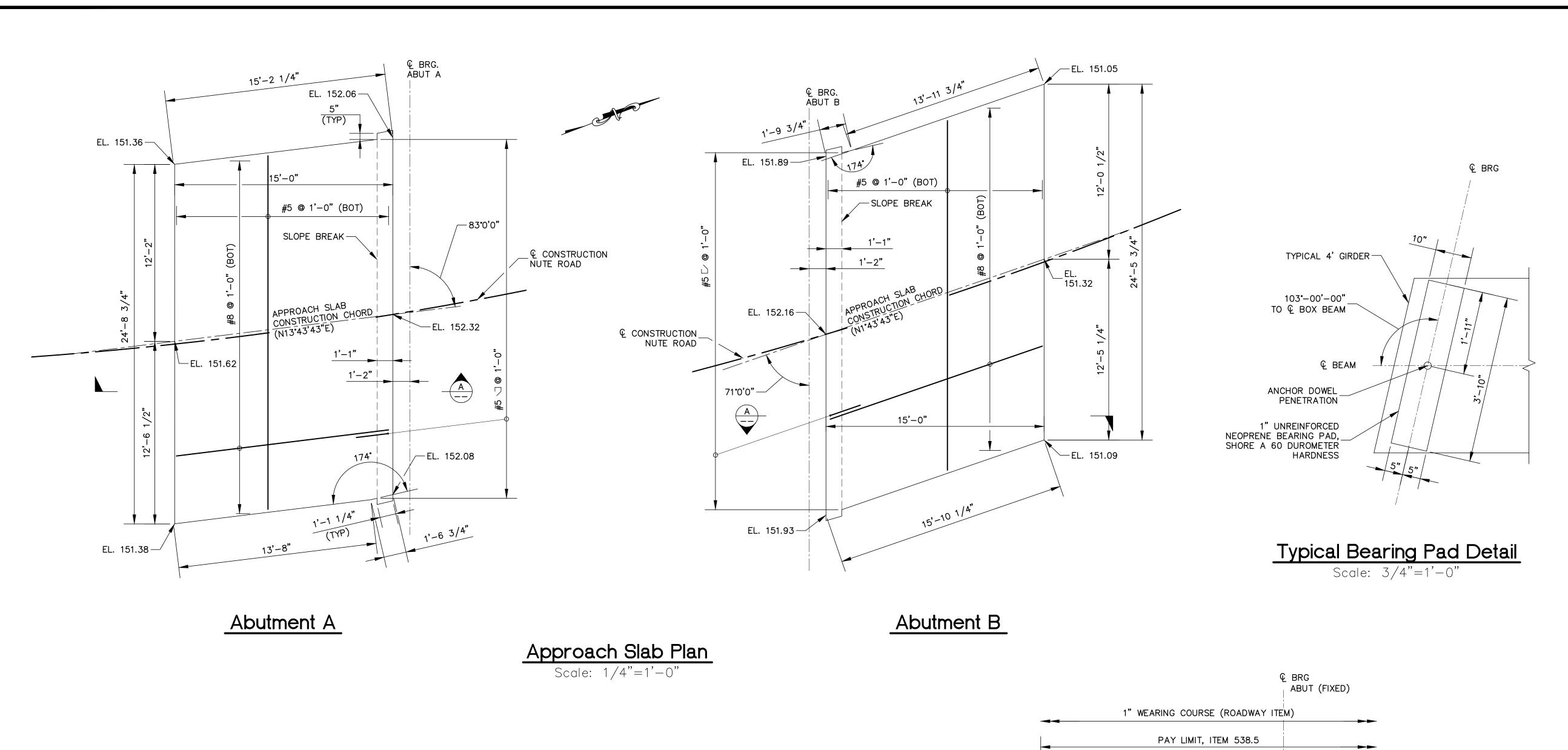
Overlay and Curb Reinforcing Plan

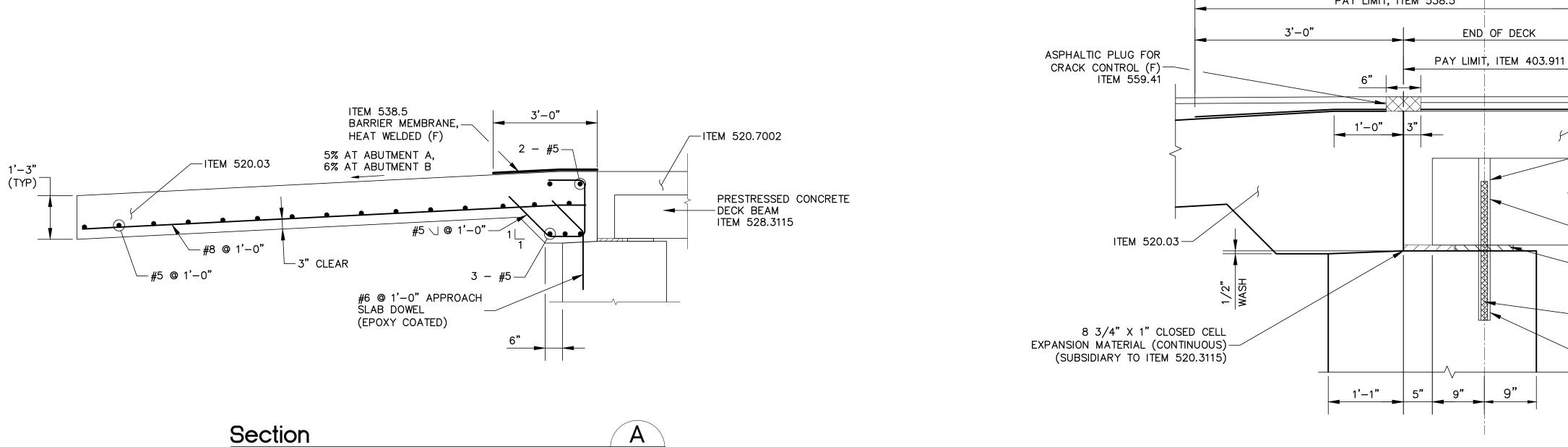




Water Repellent Limits

						PS&E Set Is:	Issued for Re	
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:de paulisap	LBK	drawn by:	TMA	approved by:		ale:	[^] 07	." = 1'-0"
date:	July 2020	project no:	1162	checked by:		scale:	0	Scale: $1/4$ " = 1'-0"
Town of Madbury, New Hampshire	•		Nute Road	-	Bellamy River Crossing Replacement		Superstructure Details	
					ng n			
she	et:		17		01	•	23	





Scale: 1/2"=1'-0"

BR-17 sheet: 18 of 23

NOT FOR CONSTRUCTION

-FINISHED GRADE

-ITEM 520.7002

FILL SLEEVE WITH HIGH — STRENGTH NON—SHRINK GROUT

(SUBSIDIARY TO ITEM 528.3115)

#9 GALVANIZED ANCHOR DOWEL

(SUBSIDIARY TO ITEM 528.3115)

2" Ø HOLE, GROUT WITH HIGH STRENGTH NON—SHRINK GROUT

(SUBSIDIARY TO ITEM 528.3115)

-BUTTED DECK BEAMS (F) ITEM 528.3115

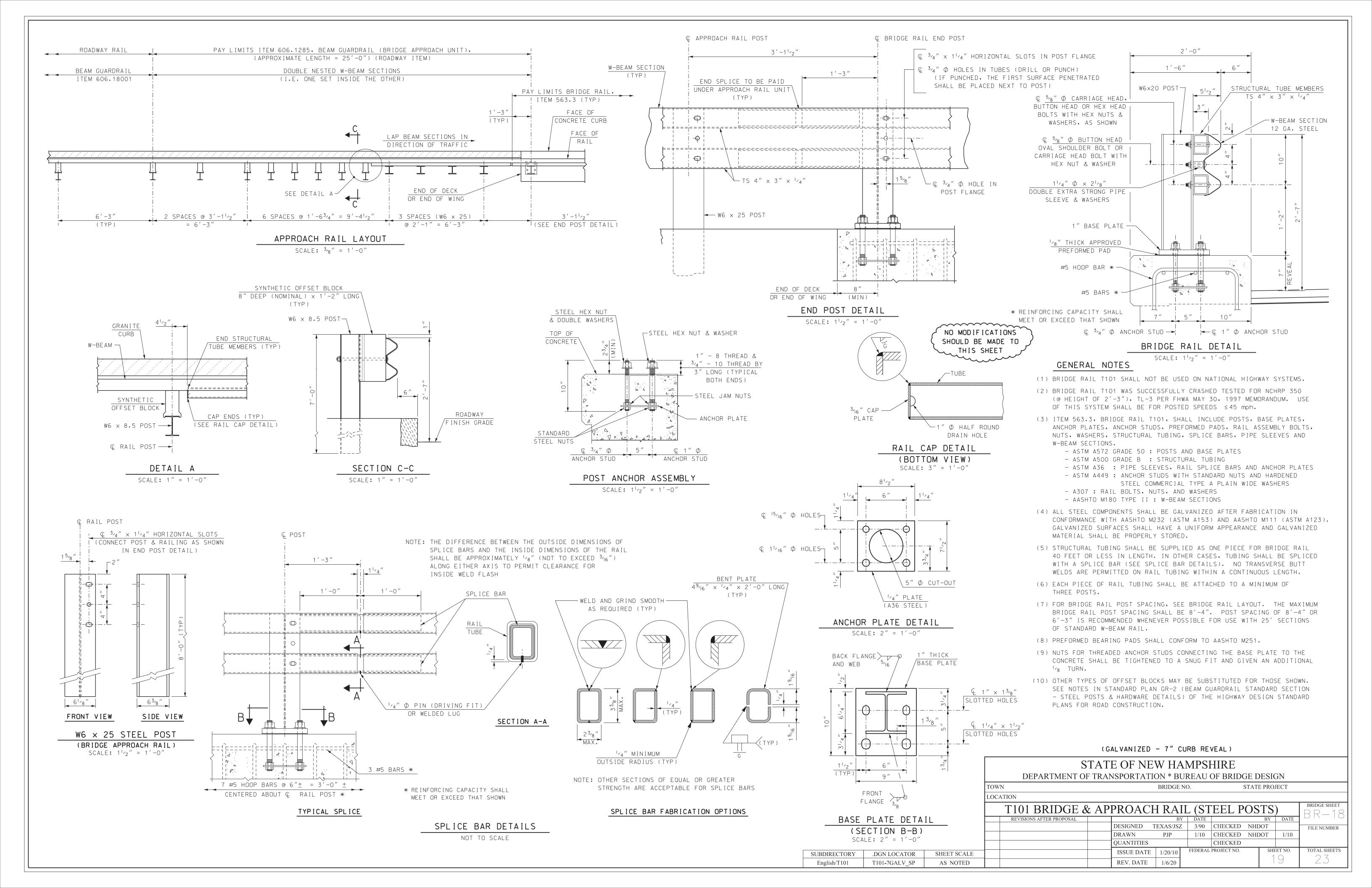
-2" ID PIPE SLEEVE

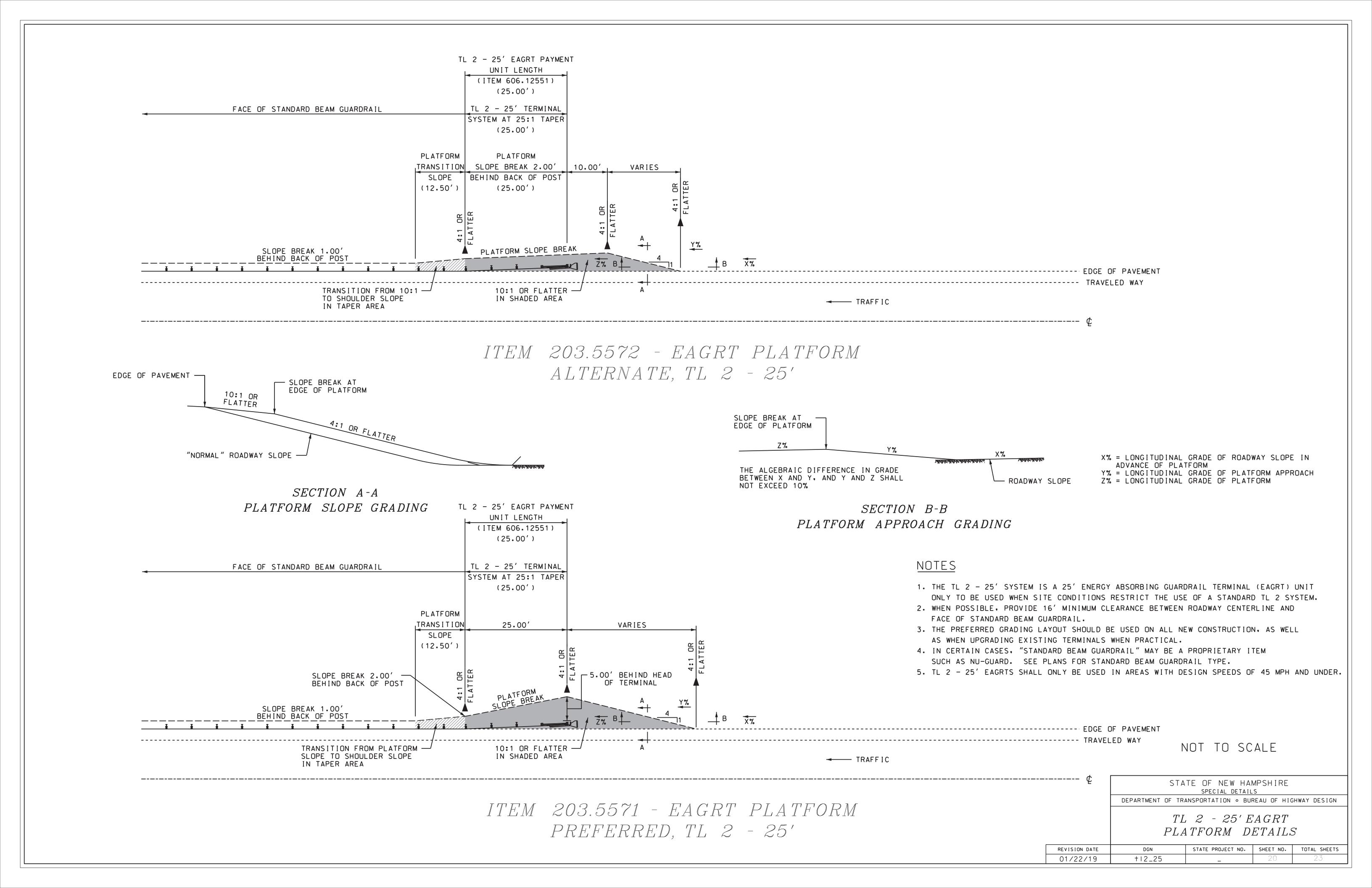
-ITEM 548.11

Typical Abutment Joint Section

Scale: 1"=1'-0"

PRESTRESSED CONCRETE BRIDGE DECK,





General Notes:

- 1) ALL WORK SHALL BE IN CONFORMANCE WITH CURRENT NHDOT STANDARD SPECIFICATIONS, DETAILS, AND THE TOWN OF MADBURY REQUIREMENTS.
- 2) ENGINEER SHALL BE DEFINED AS THE RESIDENT ENGINEER / OWNER'S REPRESENTATIVE, WHO IS RESPONSIBLE FOR ENGINEERING OBSERVATION DURING CONSTRUCTION, ACTING DIRECTLY OR THROUGH THEIR DULY AUTHORIZED REPRESENTATIVES.
- THE CONTRACTOR SHALL VERIFY ALL EXISTING UTILITY LOCATIONS PUBLIC OR PRIVATE, SHOWN OR NOT SHOWN, PRIOR TO CONSTRUCTION. THE ENGINEER SHALL BE NOTIFIED IN WRITING OF ANY UTILITIES FOUND INTERFERING WITH PROPOSED CONSTRUCTION AND THE APPROPRIATE REMEDIAL ACTION SHALL BE TAKEN BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL NOTIFY DIG-SAFE (1-800-DIG-SAFE) AT LEAST 72 HOURS PRIOR TO THE BEGINNING OF WORK TO CONFIRM THE LOCATION OF UNDERGROUND UTILITIES.
- 4) THE CONTRACTOR SHALL BE RESPONSIBLE FOR ESTABLISHING AND MAINTAINING THE HORIZONTAL AND VERTICAL CONTROL THROUGHOUT THE PROJECT.
- 5) THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING RESIDENTS OF ANY WORK RESTRICTING ACCESS TO ANY DRIVEWAY 24 HOURS IN ADVANCE.
- 6) ALL MAIL BOXES SHALL REMAIN AT THE SAME LOCATION AND BE RESET TO SAME HEIGHT IF TEMPORARY REMOVAL IS NECESSARY.
- 7) OVERHEAD UTILITY LINES ARE LOCATED THROUGHOUT THE PROJECT WITH CROSSINGS AT VARIOUS LOCATIONS AND RUNNING ALONG THE ROAD THROUGHOUT THE PROJECT. THE CONTRACTOR IS ADVISED THAT EXTREME CAUTION WILL BE REQUIRED IN THE OPERATION OF EQUIPMENT, ESPECIALLY CRANES.
- 8) REMOVE TOPSOIL FOR ITS TOTAL DEPTH WITHIN THE LIMITS OF THE SLOPE LINES. UNLESS OTHERWISE DIRECTED. STOCKPILE TOPSOIL AND USE IT ON THIS PROJECT AS NEEDED UNDER SECTION 646.51 - TURF ESTABLISHMENT WITH MULCH, TACKIFIERS AND LOAM. STOCKPILE LOCATION SHALL BE DETERMINED BY THE CONTRACTOR WITHIN THE RIGHT-OF-WAY OR OFF SITE. ALL COST FOR STOCKPILING OF TOPSOIL SHALL BE INCLUDED IN THE APPROPRIATE 646 ITEM OF THIS CONTRACT.
- 9) PERFORM ALL WORK WITHIN THE EXISTING RIGHT-OF-WAY OR ACQUIRED TEMPORARY RIGHT-OF-ENTRY LIMITS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING OR RE—ESTABLISHING ALL PROPERTY MONUMENTATION (IRON PIPE, REBAR BOUNDS, ETC.). THIS WORK SHALL BE CONSIDERED INCIDENTAL TO THE PROJECT.
- 10) CONTRACTOR SHALL PLACE BARRICADES IN A MANNER THAT WILL PROVIDE CONTINUOUS AND UNRESTRICTED ACCESS FOR RESIDENTS WITHIN THE PROJECT LIMITS.
- 11) RIGHT-OF-WAY AGREEMENTS HAVE BEEN ESTABLISHED WITH ABUTTING PROPERTY OWNERS BASED ON THE TOE OF SLOPE SHOWN ON THESE PLANS. ALL WORK SHALL BE PERFORMED WITHIN THE AREA BOUND BY THE TOE OF SLOPE. THE AGREEMENTS ARE INCLUDED AS AN APPENDIX TO THE NHDES WETLANDS PERMIT APPLICATION.
- 12) CONTRACTOR SHALL PROTECT PRIVATE PROPERTY AND SHALL TAKE ALL NECESSARY MEASURES AND PRECAUTIONS TO AVOID DAMAGE TO EXISTING TREES, SHRUBS, LAWNS, PLANTINGS, ETC. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRS/REPLACEMENT OF ALL DAMAGED ITEMS.
- 13) THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL METHODS AND MATERIALS FOR CONSTRUCTION OF THE PROJECT, INCLUDING COMPLIANCE WITH ALL APPLICABLE OSHA REGULATIONS. THE OWNER AND ENGINEER WILL PERIODICALLY REVIEW CONSTRUCTION FOR COMPLIANCE WITH THE PLANS AND SPECIFICATIONS. SUCH REVIEW DOES NOT IMPLY APPROVAL OF THE METHODS OF CONSTRUCTION.
- 14) THE CONTRACTOR SHALL EXERCISE CAUTION AND COMPLY WITH ALL APPLICABLE TRAFFIC LAWS AND REGULATIONS IN EXECUTION OF THE WORK. THE CONTRACTOR SHALL COORDINATE ACTIVITIES WITH THE TOWN'S POLICE AND FIRE DEPARTMENTS TO ENSURE ACCESS DURING CONSTRUCTION. THE CONTRACTOR SHALL FURNISH, ERECT, AND MAINTAIN BARRICADES, WARNING SIGNS, DELINEATORS, STRIPING, FLAGGERS, AND PILOT CARS IN ACCORDANCE WITH THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD) AND THE SPECIFICATIONS. THE CONTRACTOR SHALL BEAR ALL EXPENSE OF MAINTAINING THE SECTION OF ROAD UNDERGOING IMPROVEMENT INCLUDING ALL TEMPORARY APPROACHES OR CROSSINGS AND INTERSECTIONS WITH TRAILS. ROADS. STREETS. BUSINESSES. PARKING LOTS. RESIDENCES. GARAGES. FARMS, AND OTHER FEATURES THAT MAY BE NECESSARY. THE CONTRACTOR SHALL USE ALL NECESSARY MEANS TO CONTROL DUST DURING THE CONSTRUCTION PERIOD INCLUDING THE USE OF CALCIUM CHLORIDE.
- 15) ALL WORK IS TO BE COMPLETED DURING PERIODS OF LOW FLOW. THE CONTRACTOR SHALL PUMP/DIVERT STREAM FLOW AROUND WORK AREA TO MINIMIZE SILTATION IN STREAM WATERS. THE CONTRACTOR SHALL BE PREPARED FOR, AND MAKE PROVISIONS FOR, HIGH FLOW EVENTS THAT MAY OCCUR EVEN DURING TYPICAL LOW FLOW PERIODS.
- 16) THE CONTRACTOR SHALL REVIEW AND COMPLY WITH ALL APPLICABLE NOTES IN THE "PERMIT REQUIREMENT NOTES" INCLUDED ON SHEET W-1 OF THE WETLAND PERMIT PLANS INCLUDED IN THE CONTRACT DOCUMENTS.

Survey Notes:

- 1) FIELD SURVEY PERFORMED BY DOUCET SURVEY INC. BY J.P.E. & P.J.S. DURING 09/12 USING A TRIMBLE 5603 DR 200 PLUS TOTAL STATION AND BY G.A.N. & E.J.S. DURING 08/18 USING A TRIMBLE S7 TOTAL STATION WITH A TRIMBLE TSC3 DATA COLLECTOR AND A SOKKIA B21 AUTO LEVEL. TRAVERSE ADJUSTMENT BASED ON LEAST SQUARE ANALYSIS.
- 2) JURISDICTIONAL WETLANDS DELINEATED BY MARC JACOBS, CERTIFIED WETLAND SCIENTIST NUMBER 090, DURING AUGUST 2018 IN ACCORDANCE WITH 1987 CORPS OF ENGINEERS WETLANDS DELINEATIONS MANUAL, AND TECHNICAL REPORT Y-87-1.
- 3) HORIZONTAL DATUM BASED ON NHSPC (2800) NAD83 (2011) (US FEET) DERIVED FROM REDUNDANT GPS OBSERVATIONS UTILIZING THE KEYNET GPS VRS NETWORK.
- 4) VERTICAL DATUM IS BASED ON APPROXIMATE NAVD88 (GEOID12A) (±0.2') DERIVED FROM REDUNDANT GPS OBSERVATIONS UTILIZING THE KEYNET GPS VRS NETWORK.
- 5) THE ACCURACY OF MEASURED UTILITY INVERTS AND PIPE SIZES/TYPES IS SUBJECT TO NUMEROUS FIELD CONDITIONS, INCLUDING: THE ABILITY TO MAKE VISUAL OBSERVATIONS, DIRECT ACCESS TO THE VARIOUS ELEMENTS, MANHOLE CONFIGURATION, ETC.
- DUE TO THE COMPLEXITY OF RESEARCHING ROAD RECORDS AS A RESULT OF INCOMPLETE, UNORGANIZED, INCONCLUSIVE, OBLITERATED, OR LOST DOCUMENTS, THERE IS AN INHERENT UNCERTAINTY INVOLVED WHEN ATTEMPTING TO DETERMINE THE LOCATION AND WIDTH OF A ROADWAY RIGHT OF WAY. THE EXTENT OF THE RIGHT OF WAY IS NOT INCLUDED ON THESE PLANS AS THEY WERE UNABLE TO BE IDENTIFIED. THE "MUTUAL RIGHT OF WAY AGREEMENT" (S) INCLUDED IN THE PROJECT CONTRACT DOCUMENTS ARE BASED ON THE TOE OF SLOPE SHOWN ON THE "MUTUAL RIGHT OF WAY AGREEMENT PLAN" WHICH IS ALSO INCLUDED IN THE PROJECT CONTRACT DOCUMENTS. NO WORK SHALL BE PERFORMED OUTSIDE OF THIS BOUNDARY WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.
- 7) FLOOD HAZARD ZONE:"A", PER FIRM MAP #33017C0305E, DATED 9/30/15.

Roadway Notes

- THE PAVEMENT SHALL BE SUPERPAVE MIXTURE WITH PERFORMANCE GRADE 64-28 LIQUID ASPHALT. TOTAL REUSED BINDER LIMITED TO 1.0% MAXIMUM. THE VOLUMETRIC MIX DESIGN SHALL BE IN ACCORDANCE WITH NHDOT SPECIFICATIONS AND AASHTO STANDARD PRACTICE R35.
- ALL PAVEMENT EDGES SHALL BE SAWCUT. THE CONTRACTOR SHALL TRIM, TACK, AND MATCH EXISTING PAVEMENT AT LOCATIONS WHERE NEW PAVEMENT MEETS EXISTING PAVEMENT.
- 3) A TACK COAT SHALL BE APPLIED TO PAVEMENT BETWEEN COURSES AND TO ALL ABUTTING PAVEMENT SURFACES IN ACCORDANCE WITH NHDOT STANDARD SPECIFICATIONS AND SUPPLEMENTAL SPECIFICATION SECTION 410. THE TACK COAT SHALL BE SUBSIDIARY TO THE PAVEMENT COST
- 4) PAVEMENT JOINT ADHESIVE (ITEM 403.6) SHALL BE APPLIED ON ALL LONGITUDINAL PAVEMENT JOINTS FOR ALL COURSES AND AT TRANSVERSE JOINTS FOR DRIVEWAYS AND PROJECT LIMITS FOR ALL COURSES.

	PAVEMENT COURSES										
COURSE	MINIMUM LIFT THICKNESS	NOMINAL MAX. AGGREGATE SIZE	GYRATORY COMPACTION EFFORT	MINIMUM BINDER CONTENT*							
WEARING	1"	3/8"	75	6.0%							
ROAD-BINDER	3"	3/4"	50	5.1%							
BRIDGE-BINDER	1 1/2"	1/2"	50	5.1%							

*SEE ROADWAY NOTES

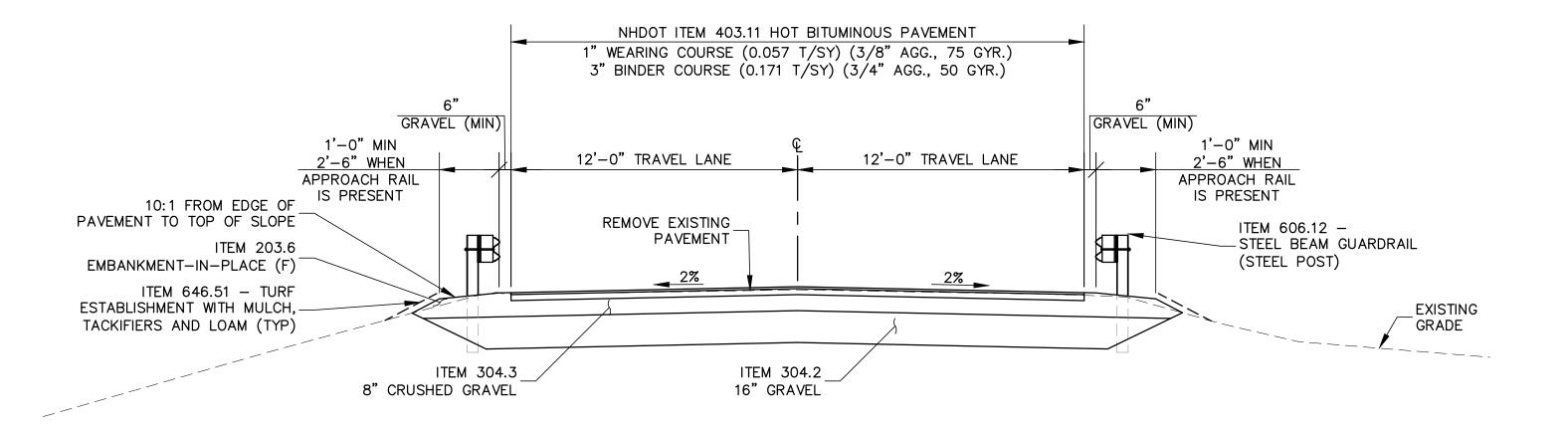
Erosion Control Notes:

- 1) THE CONTRACTOR IS RESPONSIBLE FOR THE DEVELOPMENT AND APPROVAL OF THE STORM WATER POLLUTION PREVENTION PLAN (SWPPP).
- 2) PRIOR TO CONSTRUCTION AND THEREAFTER, EROSION CONTROL MEASURES ARE TO BE IMPLEMENTED AS NECESSARY. THE SMALLEST PRACTICAL AREA OF LAND SHOULD BE EXPOSED AT ANY ONE TIME DURING CONSTRUCTION. WHEN LAND IS EXPOSED DURING CONSTRUCTION, THE EXPOSURE SHOULD BE KEPT TO THE SHORTEST PRACTICAL PERIOD OF TIME. ANY DISTURBED AREAS THAT ARE TO BE LEFT UN-STABILIZED LONGER THAN TWO WEEKS SHALL BE TEMPORARILY SEEDED AND MULCHED AT THE RATE OF 2 TONS PER ACRE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL REMEDIAL WORK REQUIRED TO REPAIR AREAS WHICH ARE DAMAGED BY EROSION.
- 3) HAY BALE BARRIERS SHALL BE INSTALLED AND MAINTAINED AT DRAIN INLETS AND OUTLETS AND ALONG LIMITS OF WORK WHERE NECESSARY. HAY BALE BARRIERS SHALL NOT BE PLACED CLOSER THAN 25-FEET TO DRAIN INLETS AND OUTLETS. ADDITIONAL HAY BALES SHALL BE ADDED AS REQUIRED BY THE ENGINEER. HAY BALES WILL BE STAKED AND MAINTAINED PRIOR TO AND DURING CONSTRUCTION UNTIL DISTURBED AREAS HAVE A HEALTHY STAND OF GRASS.
- ALL DISTURBED AREAS AND SIDE SLOPES THAT ARE AT ARE FINISH GRADED WITH NO FURTHER CONSTRUCTION TAKING PLACE SHALL BE TRACKED, SEEDED (IN ACCORDANCE WITH SECTION 644 OF THE STANDARD SPECIFICATIONS) AND MULCHED. ALL SEED, LIME AND FERTILIZER PROGRAMS SHALL CONFORM TO ALL APPLICABLE SECTIONS OF THE SPECIFICATIONS (SECTION 642 AND SECTION 643).
- 5) CONSTRUCTION TRAFFIC SHALL TRAVEL THE ROADBEDS OF EXISTING ROADS.
- SILT FENCE SHALL BE INSTALLED AND MAINTAINED WHERE NECESSARY AND ADDITIONAL SILT FENCE ADDED AS REQUIRED BY THE ENGINEER PRIOR TO ANY ON-SITE GRADING OR DISTURBANCE OF EXISTING SURFACE MATERIAL. GENERALLY, SILT FENCE SHALL BE INSTALLED TO PREVENT MIGRATION OF THE SEDIMENT FROM THE WORK AREA. IT SHOULD BE MAINTAINED DURING AND AFTER CONSTRUCTION TO REMOVE SEDIMENT FROM NATURAL DRAINAGE WAYS. THE SILT FENCE IS TO BE MAINTAINED AND CLEANED UNTIL ALL SLOPES HAVE A HEALTHY STAND OF GRASS.
- AFTER ALL DISTURBED AREAS HAVE BEEN STABILIZED, THE TEMPORARY EROSION CONTROL MEASURES ARE TO BE REMOVED AND ACCUMULATED SEDIMENT DISPOSED OF IN A LOCATION DESIGNATED BY THE OWNER.
- 8) HAY BALES AND MULCH SHALL BE MOWINGS OF ACCEPTABLE HERBACEOUS GROWTH, FREE FROM NOXIOUS WEEDS OR WOODY STEMS AND SHALL BE DRY.
- 9) SILT FENCES SHALL BE A MINIMUM OF 36 INCHES HIGH WITH THE BOTTOM OF THE CLOTH KEYING INTO THE GROUND. POSTS SHALL BE OF WOOD OR STEEL.
- 10) THE EROSION CONTROL DEVICES DESCRIBED AND AS SPECIFIED IN THE SPECIFICATIONS REPRESENT THE MINIMUM REQUIRED MEASURES FOR EROSION CONTROL. THE CONTRACTOR SHALL ADD TO THESE DEVICES ANY OTHER MEASURES AS REQUIRED OR AS DIRECTED BY THE ENGINEER TO EFFECTIVELY PREVENT MIGRATION OF SEDIMENT FROM THE WORK AREA AND PROTECT WETLAND AREAS, WATERWAYS, EXISTING AND PROPOSED DRAINAGE FEATURES. SLOPES. LAWNS. AND PLANTS ADJACENT TO THE WORK AREA.

	SUMMARY OF ROADWAY QUANTITIES	1	.
ITEM NO.	ITEM DESCRIPTION	QUANTITY	UNIT
201.1	CLEARING AND GRUBBING (F)	0.5	А
202.7	REMOVAL OF GUARDRAIL	244	LF
203.1	COMMON EXCAVATION	690	СҮ
203.6	EMBANKMENT-IN-PLACE (F)	40	СҮ
207.1	COMMON CHANNEL EXCAVATION	300	СҮ
214	FINE GRADING	1	U
304.2	GRAVEL (F)	430	СҮ
304.3	CRUSHED GRAVEL (F)	240	СҮ
403.11	HOT BITUMINOUS PAVEMENT, MACHINE METHOD	230	TON
403.12	HOT BITUMINOUS PAVEMENT, HAND METHOD	5	TON
403.6	PAVEMENT JOINT ADHESIVE	900	LF
403.911	HOT BITUMINOUS BRIDGE PAVEMENT, 1" BASE COURSE	6	TON
417	COLD PLANING BITUMINOUS SURFACES	50	SY
504.1	COMMON BRIDGE EXCAVATION (F)	550	СҮ
563.3	BRIDGE RAIL T101	120	LF
606.126	BEAM GUARDRAIL (TERMINAL UNIT TYPE EAGRT, TL 2, 25') (STEEL POST)	4	U
606.129	BEAM GUARDRAIL (BRIDGE APPROACH UNIT)	4	U
606.18	31" W-BEAM GUARDRAIL WITH 8" OFFSET BLOCK (STEEL POST)	225	LF
609.01	STRAIGHT GRANITE CURB	40	LF
619.1	MAINTENANCE OF TRAFFIC	1	U
628.2	SAWED BITUMINOUS PAVEMENT	190	LF
645.531	SILT FENCE	500	LF
645.7	STORM WATER POLLUTION PREVENTION PLAN	1	U
645.71	MONITORING SWPPP AND EROSION AND SEDIMENT CONTROLS	50	HR
646.51	TURF ESTABLISHMENT WITH MULCH, TACKIFIERS AND LOAM	240	SY
692	MOBILIZATION	1	U
1008.9	ALTERATIONS AND ADDITIONS AS NEEDED - TESTING OF MATERIALS	1	\$

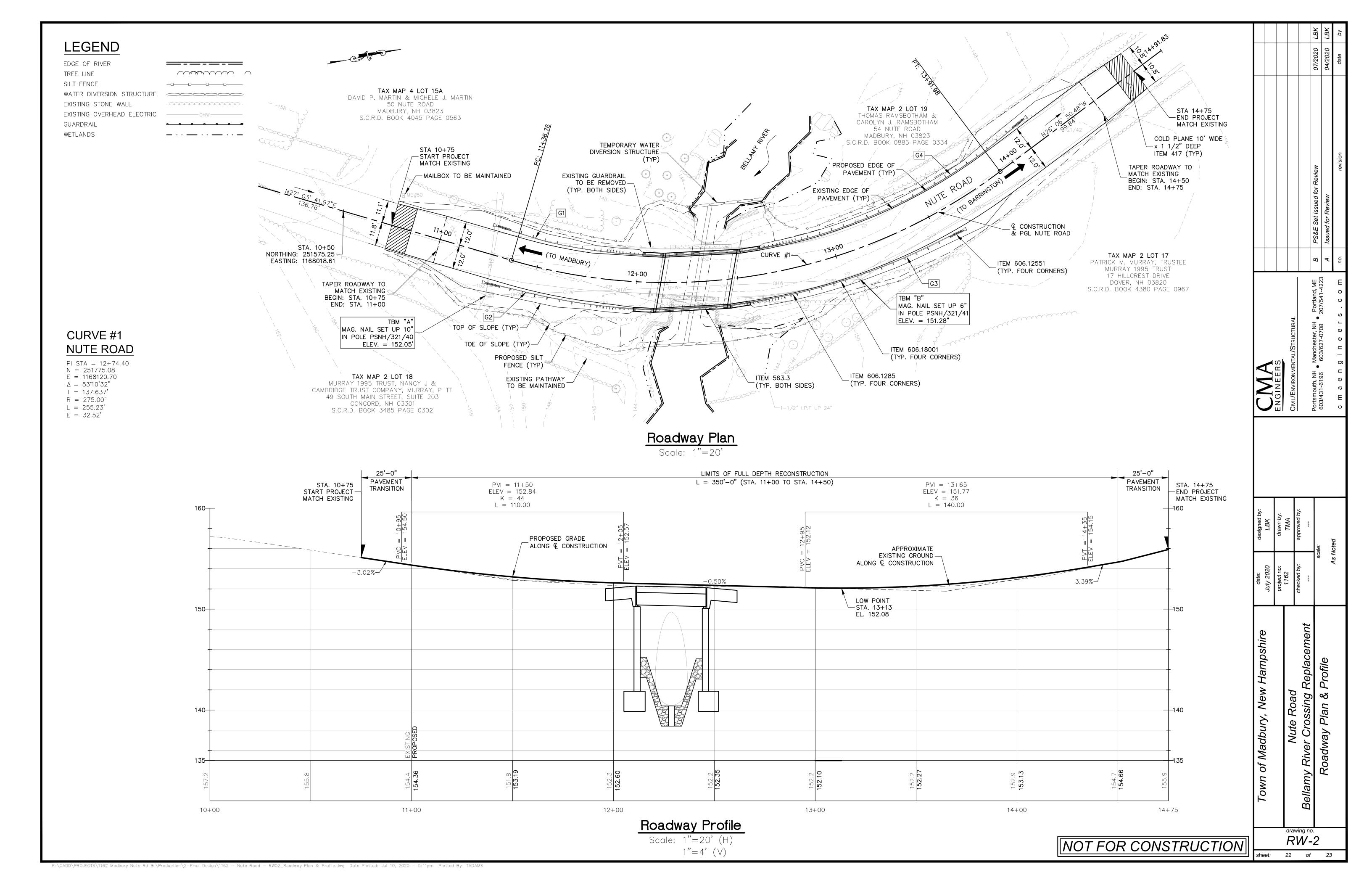
Guardrail Notes:

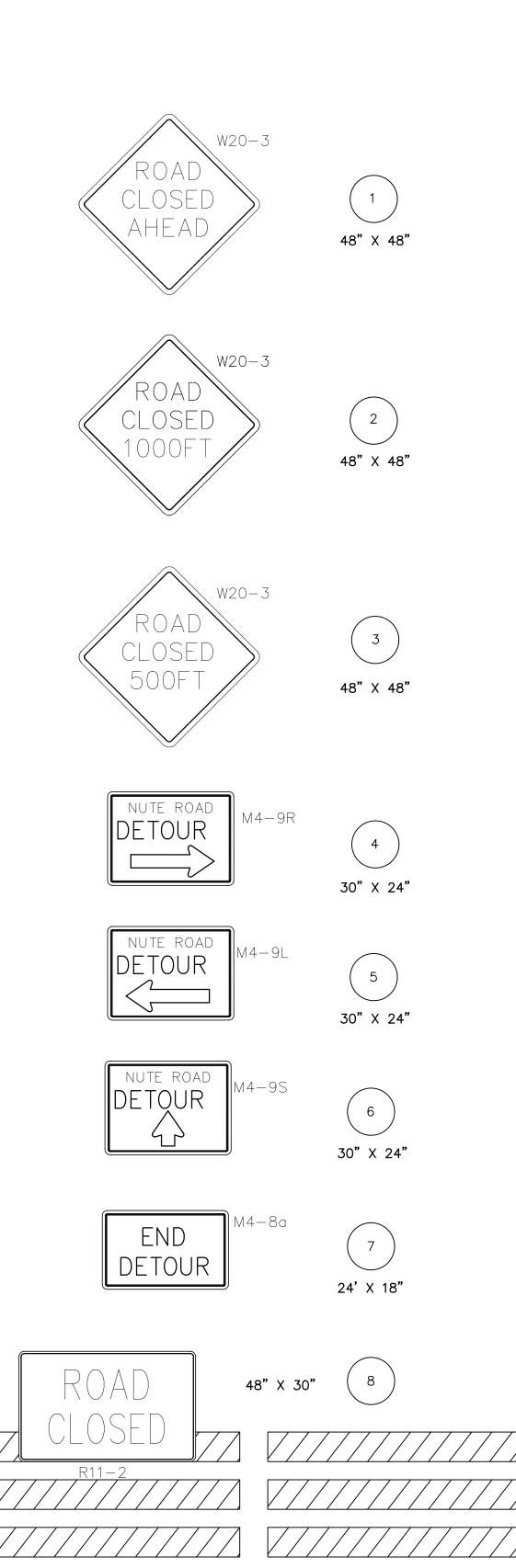
- GUARDRAIL OFFSET BLOCKS SHALL BE SYNTHETIC.
- ALL OFFSETS ARE TO FACE OF RAIL. EAGRT OFFSETS ARE TO THE FACE OF RAIL AT THE TERMINAL UNIT.
- G1 STA 12+03.3 TO 11+78.3 LT 13.3' RADIUS = 261.67CONST. T101 APPROACH RAIL STA 11+78.3 TO 11+53.3 LT 13.3' RADIUS = 261.67CONST. 25.0 LF W BEAM GUARDRAIL STA 11+53.3 TO 11+28.3 LT 13.3' AT 11+53.3 LT 14.0' AT 11+28.3 CONST. 25' EAGRT W/ PLATFORM
- G2 STA 11+99.6 TO 11+74.6 RT 13.2' RADIUS = 288.25'CONST. T101 APPROACH RAIL STA 11+74.6 TO 11+43.4 RT 13.2' RADIUS = 288.25'CONST. 31.25 LF W BEAM GUARDRAIL STA 11+43.4 TO 11+18.4 RT 13.2' AT 11+43.4 RT 14.0' AT 11+18.4 CONST. 25' EAGRT W/ PLATFORM
- G3 STA 12+53.1 TO 12+78.1 RT 12.5' RADIUS = 287.50'CONST. T101 APPROACH RAIL STA 12+78.1 TO 13+40.6 RT 12.5' RADIUS = 287.50CONST. 62.5 LF W BEAM GUARDRAIL STA 13+40.6 TO 13+65.6 RT 12.5' AT 13+40.6 RT 13.5' AT 13+65.6 CONST. 25' EAGRT W/ PLATFORM
- G4 STA 12+62.7 TO 12+87.7 LT 12.5' RADIUS = 262.50'CONST. T101 APPROACH RAIL STA 12+87.7 TO 13+94.0 LT 12.5' RADIUS = 262.50'CONST. 106.25 LF W BEAM GUARDRAIL STA 13+94.0 TO 14+19.0 LT 12.5' AT 13+94.0 LT 13.5' AT 14+19.0 CONST. 25' EAGRT W/ PLATFORM



Typical Approach Roadway Section

W W						-		
V V						Portland, ME B	A A A A A A A A A A A A A A A A A A A	s . c o m
		ENGINEERS	CIVI /ENVIRONMENTAL /STRIICTI BAL			Portsmouth, NH Manchester, NH Portland, ME 603/131-6106 603/627-0708 207/541-4223		c m a e n g i n e e r s . c o m
designed by:	LBK	drawn by:	TMA	approved by:	ı	scale:		N/A
date:	July 2020	project no:	1162	checked by:	ŀ	SS	•	Z
Towns of Modern Monday I lower	I OWIT OF MAUDULY, INEW MAINDSHIFE		Nite Road		Bellamy River Crossing Replacement		Roadway Notes	
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Detour Plan

Scale: 1"=1000

*CONTRACTOR SHALL PROVIDE GAP IN BARRICADE AND BARRIER TO MAINTAIN VEHICLE ACCESS TO DRIVEWAY

NOTES:

1. ALL TRAFFIC CONTROL DEVICES SHALL CONFORM WITH SECTIONS 618 AND 619 OF THE STANDARD SPECIFICATIONS, THESE TRAFFIC CONTROL PLANS, THE CURRENT EDITION OF THE MUTCD, INCLUDING ALL REVISIONS, AND THE STATE OF NEW HAMPSHIRE DOT TRAFFIC CONTROL HANDBOOK.

2. 30 LINEAR FT OF TEMPORARY CONCRETE BARRIER SHALL BE PLACED TO BLOCK THE ROADWAY DIRECTLY BEHIND THE TYPE III BARRICADES (#8).

3. CONTRACTOR IS RESPONSIBLE FOR SUPPLYING AND MAINTAINING ALL SIGNS REQUIRED FOR DETOUR.

4. SIGNS SHALL BE REMOVED OR COVERED DURING PERIODS IN WHICH THEY ARE NOT REQUIRED.

5. LAYOUT SHOWN IS NOT TO SCALE AND IS A SUGGESTED LAYOUT.

6. CONTRACTOR SHALL SUBMIT A SIGNAGE PLAN FOR APPROVAL PRIOR TO ANY WORK.

7. TOTAL ESTIMATED SIGN AREA: 202 SF

8. ALL COST FOR TRAFFIC CONTROL DEVICES INCLUDING PLACEMENT, RELOCATION, AND REMOVAL OF SIGNS SHALL BE INCLUDED IN ITEM 619.1 — MAINTENANCE OF TRAFFIC.

9. THE CONTRACTOR SHALL COORDINATE ALL WORK WITH THE TOWN OF MADBURY PUBLIC WORKS, FIRE AND POLICE DEPARTMENTS, AND ENGINEER AT LEAST 14 DAYS PRIOR TO IMPLEMENTING AND ROAD CLOSURES OR DETOURS.

10. ACCESS TO EXISTING DRIVES SHALL BE MAINTAINED AT ALL TIMES. IN THE EVENT THAT MAJOR WORK MUST BE DONE AT DRIVES THAT PRECLUDES FULL ACCESS, THE CONTRACTOR IS TO COORDINATE THE WORK WITH THE OWNER TO MINIMIZE INCONVENIENCE.

NOT FOR CONSTRUCTION

RW-3 sheet: 23 of 23

F:\CADD\PROJECTS\1162 Madbury Nute Rd Br\Production\2-Final Design\1162 - Nute Road - RW03_Detour Plan.dwg Date Plotted: Jul 10, 2020 - 5:12pm Plotted By: TADAMS